# Evaluation of Precipitation Network in Snowy Range Observatory

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## ABSTRACT

This research studies the monthly precipitation information content in the Snowy Range Observatory. The Observatory currently consists of 20 precipitation gages with varying length of record. The study was proceeded through three phases.

<u>Phase I - Estimation of Missing Values</u> Due to various reasons, records of all gages involves missing values. The study was first performed to compare various methods of different complexity to estimate the missing values. Although there was no single method that is universally superior in all circumstances, a simple method of linear inverse distance method was found to be rather accurate. It was then used to fill the missing values in all precipitation gages.

<u>Phase II - Analysis of Monthly Precipitation</u> The time series of monthly total precipitation was first analyzed station by station. The accuracy of different methods of estimating the spatial distribution of the average precipitation were investigated. Then, the spatial structures of the monthly total precipitation were identified using variogram analysis of geostatistics. Information derived from this phase serves as the basis for precipitation network analysis in the next phase.

<u>Phase III - Precipitation Network Analysis</u> The objective of this phase is to examine the effect of having a reduced network, in terms of gage number, on the loss of information content. The present network containing 21 gages was used as the basis for comparison. Two reduced networks each containing 15 and 12 gages, respectively, were subjectively selected on the basis of geographical location, accessibility, and aesthetic considerations. Nonstationary Kriging technique was employed to estimate the spatial distribution of error based on different network configurations. It was found that the two reduced networks have a small increase in error, as compared with 21 stations, during the months of May - September. However, increase in error could be as high as 25%-30% during the months of October - April. High error occurs on the upper third of the watershed resulting from the removal of gages from the area due to accessibility consideration.

# ACKNOWLEDGMENTS

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## CHAPTER I

#### INTRODUCTION

The Snowy Range Observatory has been maintained since the mid-1960's. Great environmental diversity is found within the relatively small geographical area. The Snowy Range, thus, has been utilized as a study for many research projects. Centered in the Nash Fork Creek drainage of the Medicine Bow National Forest in the upper North Platte River basin (Figure I.1), the Observatory consists of a network of 21 recording precipitation gaging stations and other stations measuring streamflow, humidity, temperature and wind. The list of the stations is presented on Table I.1.

From 1972 to 1988, precipitation data were collected and the various types of the gaging instruments were increased to suit the needs of the research being conducted. During the same period, data collection was interrupted in 1974-1975 which was attributed to the 'energy-boom' in Wyoming resulting in a shift of research focus from high-mountain watershed projects to environmental assessment.

Two primary objectives in this research: (1) to evaluate the existing precipitation gage network in Snowy Range Observatory and (2) to determine the reduced network which retains the maximum amount of precipitation information subject to geographical and strategical constraints.

This research analyzes the monthly precipitation data that have been stored in Water Resources Data System (WRDS) maintained by the Wyoming Water Research Center. Due to unexpected interruption in data collection system, the study first estimates missing values occurring in each station. Before missing values were estimated, various techniques were applied and their performances were examined. Statistical analysis of monthly precipitation data, which is nonstationarity, was performed which served as the basis for formulating the optimal reduced precipitation network model.

The report is organized as the following: Chapter II discusses the estimation of missing values by different methods and compares their In Chapter III, procedures are described and applied to performance. transform a nonstationary monthly precipitation time series to a stationary one. Chapter IV considers the spatial distribution of the monthly precipitation by two types of method: (1) the spatially weighted average using "Inverse Distance" and "Gaussian Smoothing" methods without considering spatial correlation, and (2) 'non-stationary kriging' considering the spatial correlation of monthly precipitation. The performance of these methods were compared on the basis of contour maps for the estimated means and the associated errors. Chapter V presents the model formulation for the optimum precipitation network design using zeroone mixed integer programming. The summary of final results and some recommendations are presented in Chapter VI. The contour maps resulting



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Station		Location		Number of
U	Latitude	Longitude	Elevation	of record
0101	41-18-00	106-09-00	8440.00	12 yrs
0102	41-20-00	106-12-00	9400.00	12 yrs
0103-1	41-21-00	106-14-00	9940.00	14 yrs
0103-2	41-21-00	106-14-00	9940.00	14 yrs
0103-A	41-21-00	106-13-00	10060.00	14 yrs
0106	41-20-00	106-11-00	9100.00	12 yrs
0108-2	41-22-00	106-15-00	10360.00	14 yrs
0108-A	41-22-00	106-15-00	10360.00	14 yrs
0109	41-22-00	106-16-00	10740.00	12 yrs
0115-2	41-22-00	106-15-00	10640.00	14 yrs
0115-A	41-22-00	106-15-00	10560.00	14 yrs
0119	41-21-00	106-13-00	9880.00	12 yrs
0120	41-21-00	106-13-00	9960.00	12 yrs
0121	41-22-00	106-14-00	10320.00	12 yrs
0121-A	41-21-49	106-13-50	10320.00	07 yrs
0122	41-21-00	106-15-00	10380.00	12 yrs
0123	41-21-00	106-15-00	10380.00	12 yrs
0124	41-21-00	106-15-00	10440.00	12 yrs
0125	41-23-00	106-15-00	10800.00	12 yrs
0126	41-22-00	106-16-00	11020.00	12 yrs
0127	41-21-00	106-13-00	9840.00	12 yrs

Table I.1 : List of Precipitation Stations in Snowy Range Observatory (Wesche,1982)

from Chapter IV and a FORTRAN program developed for the analysis are given in Appendices A and B, respectively.

# CHAPTER II

#### ESTIMATION OF MISSING DATA

#### II.1 Introduction

The precipitation data from Snowy Range Observatory had been analyzed in this study starting from June 1962 to June 1988 (a total of 169 months). There are, however, observations missing in the data set. When a designed analysis is spoiled by missing data there are basically two ways to perform the analysis. One is to analyze the observed incomplete data set. Alternatively, an approach can be applied to estimate the missing values and then to analyze the 'complete' precipitation data with these estimated values inserted. The first approach is undesirable for this study, in particular, because the missing data occurred very irregularly through the recording period in all existing stations. For example, two monthly precipitation time series containing missing data from stations 106 and 108-2, are shown in Figures II.1 and II.2.

Simultaneous time-space data are required at all stations to characterize the temporal and spatial correlation structures of the precipitations. Only 51 months throughout the entire 169 months were recorded concurrently at the Snowy Range watershed. Thus, ignoring the missing values results in losing about two thirds of the observations at all precipitation gages (on the average, each individual station out of all 21 stations has about 150 observations). For this reason, it is undesirable to conduct this study without estimating the missing values.

In the first attempt, the Box-Jenkins univariate forecasting method (Markridakis et al., 1983 and Vandaele, 1983) was used to analyze temporal correlation of monthly precipitation for each individual station. This is because the missing values occurred irregularly making it difficult to use multivariate time series analysis which requires the observations to occur concurrently.

In univariate time series analysis, the first step is to identify if the time series has a specific ARIMA structure. For this, the autocorrelation function is obtained by SAS/ETS (SAS, 1984) for each individual station. Sample results for stations 102 and 119 are shown in Figures II.3 and II.4. By using the Ljung-Box test (Bowerman et al., 1987 and Markridakis et al., 1983). The auto-correlations were not significant because  $\chi^2$  with 30 degrees of freedom is 43.77 at 5% significant level which is large than the statistic value of 39.56 for station 102 and 38.78 for station 119. This implies that the time series of monthly precipitation data for stations 102 and 119 are random with no significant temporal correlation structure. This situation occurs throughout all the remaining 19 precipitation stations in the Observatory. Since no specific ARIMA models could be identified for monthly precipitation data, the use of the Box-Jenkins forecasting method for estimating missing values was abandoned.



Precipitation (Inches) at Station 106

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Precipitation (Inches) at Station 1082

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		ARIN	1A	PF	200	CEL	DUF	ξE	( 5	STA	ΥT]	ION	I	102	2)							
				ΑU	JTC	CC	DRF	REI	'A	CIC	DNS	5										
LAG	COVARIANCE	-1	9	8	7	6	5	4	3	2	1	0	1	2	3	4	5	6	7	8	9	1
0	2.57152											**	* *	***	**:	**:	**1	**:	* *	**	**	*
1	0.053595									•				•								
2	0.0110287									•				•								
3	-0.083829	1								•	*			•								
4	0.026479									•				•								
5	0.0395429									•				•								
6	-0.42218									**	**			•								
7	0.168414	1								•		*		•								
8	0.375776									•		**	**	•								
9	-0.11497									•	*			•								
10	-0.43619									. * :	* *			•								
11	-0.196167									• •	**			•								
12	0.263037									•		**	ł	•								
13	-0.246032	1								• ;	**			•								
14	0.0974338	1								•		*		•								
15	-0.162852	1								•	*			•								
16	0.428415									•		**	* *	•								
17	-0.233829									• ;	* *			•								
18	-0.102615									•	*			•								
19	0.201029									•		**	k	•								
20	-0.19482	ł								. :	* *			•								1
21	-0.396177									. *:	* *			•								
22	0.188901								•			*		• ·								
23	0.488366								•			**	* *	*.								
24	0.35209											**	* *									
25	-0.0557214								•					•								
26	0.0913949								•			*										
27	0.0177282								•					•								
28	0.160982								•			*										
29	-0.490346								•	**:	**			•								
30	0.249222								•			**	k									
		•			~							•										

# AUTOCORRELATION CHECK FOR WHITE NOISE

TC	D CHI	Ľ			AU	JTOCORRI	ELATIONS	3	
LAC	SQUAR	RE I	OF PRO	ЭB					
6	3.32	6	0.768	0.021	0.004	-0.033	0.010	0.015	-0.164
12	12.03	12	0.444	0.065	0.146	-0.045	-0.170	-0.076	0.102
18	18.69	18	0.411	-0.096	0.038	-0.063	0.167	-0.091	-0.040
24	31.87	24	0.130	0.078	-0.076	-0.154	0.073	0.190	0.137
30	39.56	30	0.114	-0.022	0.036	0.007	0.063	-0.191	0.097

Figure II.3 : Autocorrelations at station 102

				17.7	AT	JTC			REI	LA	FIOI	NS
LAG	COVARIANCE	-1	9	8	7	6	5	4	3	2	1 (	0 1 2 3 4 5 6 7 8 9 1
0	2.73073	!	-	-	•	-	-	-				*****
1	-0.0133301									•		•
2	-0.249693										**	
3	0.0658215	1								•		
4	-0.0275677									•		•
5	-0.0601951									•		
6	-0.363533									•	* * *	
7	-0.0297245									•		•
8	0.433612									•		*** .
9	-0.124588									•	*	•
10	-0.206754									•	**	•
11	-0.18758									•	*	•
12	0.287082									•		** •
13	-0.186568									•	*	•
14	0.19402									•		* -
15	-0.121405									•	*	•
16	0.324022									•		** •
17	-0.155573	1								•	*	•
18	-0.248737	i								•	**	•
19	0.100324									•		* •
20	-0.210515									•	**	•
21	-0.515838	i								• *	***	• •
22	0.0325981									•		
23	0.654807									•		****
24	0.634183									•		****
25	-0.308912	i								•	**	•
26	-0.133387								•		*	•
27	-0.257144	i							•		**	•
28	0.177734	i							•			* •
29	-0.589677	i i							•	*	***	•
30	0.127712	ł				~			•			* •

ARIMA PROCEDURE (STATION 119)

# AUTOCORRELATION CHECK FOR WHITE NOISE

то	CHI				AUTO	CORREL	ATIONS		
LAG	SQUARI	E DI	F PROB						
6	2.43	6	0.876	-0.005	-0.091	0.024	-0.010	-0.022	-0.133
12	7.08	12	0.852	-0.011	0.159	-0.046	-0.076	-0.069	0.105
18	10.97	18	0.896	-0.068	0.071	-0.044	0.119	-0.057	-0.091
24	28.94	24	0.222	0.037	-0.077	-0.189	0.012	0.240	0.232
30	38.78	30	0.131	-0.113	-0.049	-0.094	0.065	-0.216	0.043

Figure II.4 :Autocorrelations at station 119

Alternatively, several estimation methods considering spatial correlation were used to estimate the missing observations. The missing data at a station can be estimated by the weighted average of the observed precipitations from the surrounding stations as

$$\hat{X}(0,t) = \sum_{s=1}^{S} w_s X(s,t)$$
 (II.1)

where X(0,t) is the estimated precipitation amount for a station with missing data at time t, X(s,t) is the observed precipitation from a surrounding station s at time t,  $w_s$  is the weight for station s, and S is the total number of surrounding stations used in estimation.

Three types of method are used to in this study estimate the missing values: (1) inverse distance weighing technique, (2) nonlinear programming technique to minimize the variance of the estimates, and (3) regression technique considering the cross-correlations between the station with missing values and the surrounding stations with observations. The resulting regression statistics such as variance, standard error, and  $R^2$  were also used to define the weights for estimating the missing values.

## II.2 Estimation Methods

Since the estimates are a weighted linear combination of the observations from the surrounding stations, the results of estimation depend on how the weighing factors are calculated.

# II.2.1 Inverse Distance Weighing Method

The inverse distance weighing method considers the premise that weight contributed from a station with observations to the estimated precipitation amount at the station with missing values is inversely proportional to the physical distance between the two stations. The contributing weight for the station s with observation to the station with missing values,  $w_s$ , is computed by

 $w_{s} = \frac{\left(\frac{1}{D_{s}^{n}}\right)}{\sum_{s=1}^{S} \left(\frac{1}{D_{s}^{n}}\right)}$ 

(II.2)

where  $D_s$  is the distance between the station s with observations and the station with missing values, n is a constant by which the distance is weighted, S is the number of surrounding stations used (Tung, 1983) in the estimation. If the value of  $\alpha$  is very large, the weights will be concen-

trated on a few stations that are very close to the station with missing values. On the other hand, the weight may be dispersed to a large number of the surrounding stations for a small  $\alpha$ . For instance, if  $\alpha$  equals zero, then every surrounding station under consideration would have an equal weight.

In this study, the values  $\alpha=1$  and 2 are used and they are called the linear inverse distance (IDLIN) method and square inverse distance (IDSQ) method, respectively. For these two methods, the IDSQ technique gives higher weight for the station closer to the point of estimation than the IDLIN method. The amount of monthly precipitation at the station with missing values can be determined by the IDLIN method as

$$\hat{X}(0,t) = \frac{\sum_{s=1}^{S} \left(\frac{X(s,t)}{D_s}\right)}{\sum_{s=1}^{S} \left(\frac{1}{D_s}\right)}$$
(II.3)

and by the IDSQ method as

$$\hat{X}(0,t) = \frac{\sum_{s=1}^{S} \left(\frac{X(s,t)}{D_{s}^{2}}\right)}{\sum_{s=1}^{S} \left(\frac{1}{D_{s}^{2}}\right)}$$
(II.4)

# II.2.2 Optimal Weighing Method

Referring to Eq.(II.1), X(0,t) is a random variable because it is a linear function of random observations of the surrounding stations. Its degree of uncertainty, represented by the variance, can be computed as

$$\sigma_{0,t}^{2} = \mathbf{E} \{ [\hat{X}(0,t) - \mathbf{E}(\hat{X}(0,t))]^{2} \} = \mathbf{E} \{ [\hat{X}(0,t) - \sum_{s=1}^{S} w_{s} \mathbf{E}(X(s,t))]^{2} \}$$

$$= \mathbf{E} \{ [\hat{X}(0, t) - \sum_{s=1}^{S} w_{s} \mu(s, t)]^{2} \} = \mathbf{E} \{ [\hat{X}(0, t) - \mathbf{w}^{T} \mu(t)]^{2} \}$$

$$= \mathbf{E} \{ [\mathbf{w}^{T} \mathbf{X}(t) - \mathbf{w}^{T} \boldsymbol{\mu}(t)]^{2} \} = \mathbf{E} \{ \mathbf{w}^{T} [\mathbf{X}(t) - \boldsymbol{\mu}(t)]^{T} [\mathbf{X}(t) - \boldsymbol{\mu}(t)] \mathbf{w} \}$$

$$= \mathbf{w}^{T} \mathbb{C}_{t} \mathbf{w}$$
(II.5)

in which

$$C_{t} = E\{[X(t) - \mu(t)]^{T}[X(t) - \mu(t)]\}, \qquad (II.6)$$

$$\hat{X}(0,t) = \sum_{s=1}^{S} w_s X(s,t) , \qquad (II.7)$$

$$\mu(s,t) = \mathbb{E}[X(s,t)]$$
(II.8)

with  $C_t$  being the covariance matrix between stations for month t;  $X_t$  and  $\mu_t$  are Sxl vectors of the random observation and the mean of monthly precipitation at station s, for s=1,2,...,S, in month t and T is transpose of a vector or a matrix. It is, then, desirable to find the weighing factors that minimizes the variance associated with the estimator X(0,t).

This method is called herein the optimum weighing method (OPTIM) in that it has the object function of minimizing the error variance of the estimator subject to the constraint that the sum of weights is 1. That is,

minimize 
$$\sigma^2_{0t}$$
 (II.9)

subject to

$$\sum_{s=1}^{S} w_s = 1$$
 (II.10)

in which  $\mathcal{L}(\mathbf{w},\lambda)$  is the Lagrange function and  $\lambda$  is the Lagrangian multiplier. The above minimization problem is solved by the Lagrange multiplier method which converts the original constrained minimization problem into an unconstrained minimization one as

min 
$$\mathscr{L}(\boldsymbol{w},\lambda) = \boldsymbol{w}^T \mathbb{C}_+ \boldsymbol{w} + 2\lambda (\boldsymbol{w}^T \mathbf{1} - 1)$$
 (II.11)

Then, the solution to Eq.(II.11) must satisfy the following equations

$$\frac{\partial \mathcal{Q}}{\partial \boldsymbol{w}} = 2\mathbb{C}_{t} \boldsymbol{w} + 2\lambda \boldsymbol{1} = \boldsymbol{0}$$
 (II.12)

$$\frac{\partial \mathcal{L}}{\partial \lambda} = \mathbf{1}^T \mathbf{w} - \mathbf{1} = 0 \qquad (II.13)$$

where 1 is a vector of ones. Solving Eqs.(II.12) and (II.13), the optimal weighing factors can be obtained as

$$\begin{pmatrix} \mathbf{w}^{\star} \\ \lambda^{\star} \end{pmatrix} = \begin{pmatrix} \mathbb{C} & \mathbf{1} \\ \mathbf{1}^{T} & \mathbf{0} \end{pmatrix}^{-1} \begin{pmatrix} \mathbf{0} \\ \mathbf{1} \end{pmatrix}$$
(II.14)

in which  $w^*$  is the vector of optimum weight for the observed stations. Then the missing values can be estimated by

$$\hat{X}(0,t) = \sum_{s=1}^{S} w_s^* X(s,t)$$
 (II.15)

in which  $w_s^*$  is the optimum weight for the observed station s.

#### II.2.3 Cross Correlation Weighing (RSQR) Method

Since the monthly precipitation could exhibit spatial correlation, measures of correlation between existing stations can be used to compute the weights. The basic idea is that if a measured station s is strongly correlated to the station with missing values through the whole recording period, then the station s should be given higher weight. Specifically, the weight for station s,  $w_s$ , by the RSQR method is computed as

$$w_{s} = \frac{r_{0s}^{2}}{\sum_{s=1}^{S} r_{0s}^{2}}$$
(II.16)

where  $r_{os}^2$  is the coefficient of determination between the station with missing values and the station s with observations.

# II.2.4 Variance (or Standard Deviation) Weighing Method

Based on the results of simple regression analysis between two stations, the statistics, such as standard error or its squared value, can also be used to compute the weight. Intuitively, the larger the variance or the standard error for the station, the smaller the weight should be assigned to it. Therefore, the weight should be inversely proportional to the variance and standard error. The inverse variance (INVAR) and inverse standard error (INSTD) techniques are similar to the IDSQ and IDLIN methods, respectively. The only difference is that the IDLIN and the IDSQ techniques require only physical distances between stations to calculate the weights whereas the INVAR and the INSTD methods require the observed monthly precipitation data to compute the statistics to obtain the weights for all stations used. The weights by the INVAR method are calculated as

$$w_{s} = \frac{\frac{1}{\sigma_{0s}^{2}}}{\sum_{s=1}^{S} \frac{1}{\sigma_{0s}^{2}}} , s = 1, 2, \dots S$$
(II.17)

and by the INSTD method

$$W_{s} = \frac{\frac{1}{\sigma_{0s}}}{\sum_{s=1}^{S} \frac{1}{\sigma_{0s}}} , \quad s = 1, 2, \dots, S \quad (II.18)$$

where  $\sigma_{0s}$  is standard error from regression between the station with missing values and station s with observations.

# II.2.5 <u>Hybrid Method</u>

In the above methods, the missing monthly precipitation data are estimated from the weighted average of the surrounding observed values using various weighing techniques. In these methods, the actual observed monthly precipitation data from the surrounding stations are used. Alternatively, simple regression equations can be developed for the station to be estimated and all other surrounding stations based on the data concurrently available. These regression equations can be used, as the first attempt, to estimate the missing value from each surrounding station. The resulting estimated missing values from the regression equations are further weighted by the above methods. That is,

$$\tilde{X}(0,t) = \frac{\sum_{s=1}^{S} \hat{X}(0,t) w_{s}}{\sum_{s=1}^{S} w_{s}}$$
(II.19)

where  $\hat{X}(0,t)$  is the missing monthly total precipitation estimated by the regression equation from station s with observations and  $\tilde{X}(0,t)$  is the estimated missing monthly total precipitation computed by the weighted average of regression estimates, X(0,t).

# II.3 Comparison Study for the Performance of Estimation Methods

Among the estimation methods, it is desirable to examine the performance of each method and identify the most accurate one. Two criteria are used in this performance evaluation and they are the root-mean-square-error (RMSE)

$$RMSE_{s,k} = \sqrt{\frac{\sum_{t=1}^{T} [X(s,t) - \hat{X}_{k}(s,t)]^{2}}{T}}, s = 1, 2, \dots S$$
(II.20)

and the mean-absolute-error (MAE)

$$MAE_{s,k} = \frac{\sum_{t=1}^{T} |X(s,t) - \hat{X}_{k}(s,t)|}{T} , s = 1, 2, \dots S$$
(II.21)

in which T is total months used, and  $\hat{X}_k(s,t)$  is the value estimated by weighing method k.

In this comparison, precipitation stations were selected under the condition that there exists, among them, as many concurrently observed values as possible and that all of them are not too close to each other. Nine such stations were chosen and they were 102, 103-1, 108-2, 115-2, 119, 122, 123, 124 and 125. Three different periods data set observed concurrently at all 9 stations were selected to establish regression equations between two stations to compute the corresponding correlation, variance, or standard error for purpose of determining the weights from different weighing techniques. These periods were a 12-month period (7/1982-12/1983), a 24-month period (2/1981-12/1983), and a 48-month period (8/1978-12/1983). Then it was assumed that the observed monthly precipitation (1/1985-7/1988) for these nine stations were missing. The estimated values for the assumed missing period by the different estimation methods on the basis of different record lengths (i.e., 12-, 24-, and 48-month) were compared with the actual observations to calculate the RMSE and MAE.

## II.4 Results and Discussion

The results associated with two stations (125, 103-1) out of 9 are presented here. Tables II.1-II.2 show the results of RMSE and MAE including the individual weight and regression equation for each station used. The values of error criteria are also plotted in Figures II.5 and II.6.

Table II.1 presents the comparison of each weighing technique for station 125. Referring to Table II.1(b), the IDLIN method estimates, without using regression, has the smallest values of RMSE (0.592) and MAE (0.436) for all three different periods. The IDSQ method has the second smallest values of errors (RMSE = 0.626, MAE = 0.456). The INVAR technique, considering regression, yields the largest values of RMSE (0.983) and MAE (0.735) for a 12-month period.

Different results for station 103-1 in Table II.2 show that the INSTD method considering regression has the smallest value of RMSE (0.348) but a slightly large MAE (.269) for a 12-month period whereas the IDSQ method, without considering regression, has the smallest value of MAE (0.255) and a rather large RMSE (0.398). Not presented here, the results for station 123 shows that the OPTIM method without using regression has the smallest values of RMSE (0.570) and MAE (0.389). It was also found that when the number of years used to establish regression equations increased, the corresponding errors decreased.

There is no single estimation technique that is uniformly superior in all circumstances. Overall speaking, the linear inverse distance weighing method (IDLIN) is better than all other methods throughout the 9 stations. The IDLIN method is then used to estimate the missing monthly precipitation for further analysis.

Precipitation Data from Snowy Range

Station to be	e esti	imated	: 125
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Stations used : 102,1031,1082,1152,

$(\lambda)$ .	Weights	
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119,122,123,124

Statio	n		Weig	ghts for	r each	Station	s		
Hethod	s	STA 102	STA 1031	STA1082	<b>STA 1152</b>	STA 119	STA 122	STA 123	STA 124
IDSQ		0.014	0.040	0.181	0.510	0.027	0.098	0.052	0.077
IDLIN		0.049	0.082	0.174	0.292	0.067	0.128	0.093	0.114
	l yrs	0.100	0.120	0.131	0.133	0.100	0.165	0.137	0.114
INSTO	2 "	0.092	0.112	0.151	0.161	0.088	0.152	0.128	0.117
	4 "	0.097	0.111	0.151	0.168	0.087	0.149	0.118	0.118
	1 "	0.115	0.127	0.132	0.125	0.118	0.136	0.120	0.128
RSQR	2 "	0.100	0.118	0.147	0.136	0.105	0.135	0.126	0.133
	4	0.108	0.118	0.142	0.135	0.108 .	0.135	0.126	0.128
	1 "	0.132	0.128	0.085	0.094	0.149	0.127	0.163	0.122
INVAR	2 "	0.159	0.137	0.079	0.091	0.162	0.121	0.138	0.112
	4 **	0.154	0.138	0.082	0.091	0.164	0.116	0.133	0.122
	1 "	0.116	0.132	0.064	0.069	0.156	0.150	0.192	0.121
OPTIM	2 "	0.136	0.138	0.072	0.082	0.152	0.141	0.160	0.119
•	4 "	• 0.146	0.140	0.071	0.078	0.164	0.128	0.146	0.127

(B). Estimation without using Regression from Individual Station

	Error			Methods		_	
Years	Criterion	IDSQ	IDLIN	INSTR	RSQR	INVAR	OPTIM
	RMSE	0.626	0.592	0.698	0.719	0.799	0.834
1	MAE	0.456	0.436	0.482	0.490	0.530	0.556
2	RMSE	0.626	0.592	0.664	0.692	0.821	0.821
-	MAE	0.456	0.436	0.470	0.481	0.541	0.545
4	RMSE	0.626	0.592	0.660	0.699	0.820	0.838
	MAE	0.456	0.436	0.468	0.482	0.540	0.553

Table II.1 : Comparison of Estimation Method for Station 125

. . . . . .

Estimation using Regression from Individual Station <u>:</u>

Years	Error	4	y Inc	tividué	al St	ation				By	. Wele	shted	Resul	t s	
	Criterion	STA 10	STA 03	STA1 08	s1 <sup>1152</sup>	STAL19	STA 122	STA123	STA124	psai	IDI.IN	INSTA	RSQR	INVAR	OPTIM
-	RMSE	1.293	1.076	.989	.974	1.297	.783	. 944	1.138	.933	• 93B	.948	.972	.983	176.
	MAE	.985	. 803	90Y .	.692	. 977	.626	. 655	.808	.677	.689	.708	.723	.735	.730
,	RMSE	1.283	1.060	.784	. 737	1.347	.781	.923	1.007	.755	808.	.855	.875	.946	.933
•	MAE	1.076	. 902	.566	.523	1.131	.563	. 690	.769	.558	.621	.681	. 700	.771	.760
	RHSE	1.110	.972	717.	.645	1.245	.724	.915	.913	.665	.717	.764	.786	.847	.850
r	MAE	166.	.841	.531	.489	1.035	.525	.712	.718	. 525	.583	.626	.644	. 697	.698
												-			

II.1

Table

\*\* Note \*\*

SE = 2.461 (0.586)\* SĘ = 1,723 (0.312) SE =1.486 (0.778) SE = .999 (0.517) 1. SÉ = 1.102 (0.425) .641 (0.665) SE = 1.791 (0.606) SE = 1.181 (0.453) SE = 2.159 (0.685) SE . . 809 (0.577) SE = 1,288 (0.508) SE = .808 (0.592) s SE = ---.907 X<sub>124</sub> 1 .825 X<sub>124</sub> .914 X<sub>122</sub> •904 X<sub>122</sub> .913 X<sub>123</sub> .902 X<sub>123</sub> .959 X<sub>123</sub> •809 X<sub>119</sub> .963 X<sub>122</sub> Y<sub>125,1</sub> \* ...724 + .770 X<sub>124</sub>  $Y_{125,2} = 1.640 + .728 X_{119}^{1.0}$  $Y_{125,4} = 1.251 + .871 X_{119}$ Y<sub>125,2</sub> = .829 + . Y<sub>125,4</sub> = .726 + . Y<sub>125,1</sub> = 1.139 + + + 29. + 187. + 673. • 546 + .370 + .395 + ۲<sub>125</sub>,1 -Y<sub>125,1</sub> Y<sub>125,2</sub> = Y<sub>125,4</sub> " Y<sub>125,2</sub> " Y<sub>125,4</sub> " (0.561)\* (0.287) (0.427) (0.675) (968.0) (0.510) (0.733) (0.617) (0.730) (0.654) (0.530) (0.659) SE = 2.454 SE = .982 SE = 2,041 SE = 1.356 SE = .860 SE = 1.876 SE = 1.003 SE = .634 SE = 1.847 ŚE = "643 .749 X<sub>1152</sub> ; SE . .571 SE #1.642 .569 X<sub>1152</sub> ; 1001 X 667. •884 X<sub>1031</sub> • • 579 X<sub>1082</sub> 1 .752 X<sub>1082</sub> 1 .784 X<sub>1031</sub> 1 .708 X<sub>1082</sub> 4 .696 X<sub>1152</sub> .684 X<sub>102</sub> .661 X<sub>102</sub> .824 X<sub>102</sub> + 617. .786 + Y<sub>125,4</sub> = 1.261 + - 740 + + 464. Y<sub>125,1</sub> = 1.138 +  $Y_{125,2} = 1.093 +$ Y<sub>125,4</sub> = .374 + Y<sub>125,2</sub> = 1.682 + Y<sub>125,4</sub> = .881 + .748 + .384 + ۲<sub>125,</sub>۱ <sup>=</sup> Y<sub>125,2</sub> " ۲<sub>125,2</sub> " ۲<sub>125</sub>,۱ " ۲<sub>125</sub>,۱ = Y<sub>125,4</sub> "

represents: R-Square

\*

Precipitation Data from Snowy Range

Station to be estimated : 1031

Stations used : 102,1082,1152,119,

(A). Weights

122,123,124,125

Statio	<u>n</u>		Weig	ghts for	c each	Station	s		
Method	s	STA102	STA1082	STA1152	STA119	STA 122	STA123	STA 124	STA 125
IDSQ		0.066	0.120	0.060	0.328	0.192	0.128	0.072	0.035
IDLIN		0.096	0.129	0.092	0.214	0.164	0,134	0.101	0.070
	l yrs	0.155	0.112	0.157	0.104	0.100	0.088	0.194	0.090
INSTO	2 "	0.158	0.102	0.143	0.095	0.130	0.107	0.186	0.078
	4 "	0.161	0.096	0.137	0.091	0.125	0.116	0.182	0,091
	1 "	0.128	0.120	0.121	0.134	0.119	0.133	0.134	0.111
RSQR	2 "	0.133	0.123	0.125	0.138	0.120	0.136	0.137	0.088
	4 ••	0.131	0.123	0.125	0.134	0.123	0.133	0.133	0.098
	1 "	0.119	0.094	0.097	0.132	0.145	0.147	0.116	0.150
INVAR	2 "	0.126	0.099	0.104	0.128	0.142	0.133	0.115	0.154
	4 "	0.133	0.099	0.102	0.138	0.133	0.131	0.122	0.141
	1 "	0.116	0.063	0.069	0.155	0.149	0.191	0.120	0.139
OPTIM	2 "	0.143	0.076	0.086	0.159	0.148	0.168	0.126	0.094
· ·	4 "	0.153	0.075	0.082	0.172	0.134	0.153	0.134	0.096

(B). Estimation without using Regression from Individual Station

	Error			Methods			
Years	Criterion	IDSQ	IDLIN	INSTD	RSQR	INVAR	OPTIM
,	RMSE	0.398	0.439	0.481	0.482	0.472	0.413
L	MAE	0.255	0.311	0.370	0.361	0.349	0.298
2	RMSE	0.398	0.439	0.462	0.470	0.483	0.401
_	MAE	0.255	0.311	0.355	0.355	0.358	. 201
4	RMSE	0.398	0.439	0.461	0.479	0.405	. 388
	MAE	0.255	0.311	0.353	0.361	0.345	. 280

Table II.2 : Comparison of Estimation Method for Station 103-1

Estimation using Regression from Individual Station : :

												•			
Years	Error	6	y Inc	lividue	al St	ation				B)	Help	thted	Reaul	t e	
	Criterion	STA102	stÅ <sup>082</sup>	st <sup>152</sup>	STAILS	STA122	STA123	STA124	STA 125	IDSQ	IDI.IN	INSTR	RSQR	INVAR	OPTIM
ı	RMSE	.499	. 692	.491	.743	171.	.878	.398	.855	.396	.368	.348	.357	.374	.366
	MAE	496.	. 547	. 396	.419	.632	.606	.258	. 599	.256	.267	.269	.274	.280	.264
2	RMSE	.444	. 690	494.	.740	.540	. 657	.378	.904	.397	.376	.361	.364	.389	.356
1	MAE	.357	.540	.387	.438	.367	.472	.274	.723	.279	.284	.284	. 285	.304	. 275
	RMSE	.418	. 698	.490	.739	.537	.580	.370	.739	.397	.375	. 360	.368	.376	.356
;	HAE	.324	.529	.388	.427	.367	.413	.259	.613	.277	.283	.281	.286	.293	.274

\*\* Note \*\*

(0.771)\* (0.732) (0.804) (0.965)

SE = .475

(0.945) (6:630)

1031,1 = 3:5 1 2:5 X 122 1 3:5 = 1,150 X 1031,2 = .283 + .863 X 122 1 3:5 = .691 X 2 - . . . Y<sub>1031,1</sub> =-.640 + 1.185X<sub>123</sub> ; SE = 1.666 Y<sub>1031,2</sub> = 1.528+ .505 X<sub>125</sub> i SE = 1.156 Y<sub>1031,1</sub> = .492 + .845 X<sub>125</sub> i SE = 1.623 Y<sub>1031,4</sub> = -.088+ .988 X<sub>123</sub> 1 SE = .513 Y<sub>1031,2</sub> =-.286 + 1.045X<sub>123</sub> ; SE = .840 Y<sub>1031,1</sub> =-.019 + .948 X<sub>124</sub> ; SE = .756 Y<sub>1031,4</sub> = .124 + .926 X<sub>124</sub> i SE = .327 Y<sub>1031,4</sub> # .165 + .856 X<sub>122</sub> + <sup>1</sup>1031,4 = .906 + .577 X<sub>125</sub> Y<sub>1031,2</sub> = .185 + .907 X<sub>124</sub> ; \*(768.0) (0.897) (0.912) (0.791) (0.770) (0.803) (0.804) (0.794) (0.823) (0.966) (926.0) (0.977) SE = 1.313 SE = 1.410 <sup>1</sup><sub>1031,4</sub> <sup>±</sup> .<sup>323</sup> + .<sup>973</sup> X<sub>102</sub> i SE = .370 SE = .947 SE = .947 SE = .617 SE = ,632 Y<sub>1031</sub>,4 = .244 + 1.056X<sub>119</sub> t SE = .654 SE = .932 SE = .433 SE =.883  $Y_{1031,2} = .450 + .938 X_{102}$  i SE =.569  $x_{1031,1} = .456 + .888 x_{102}$ Y<sub>1031</sub>,1 = .254 + 1.074X<sub>119</sub> ;  $Y_{1031,2} = .403 + 1.028X_{119}$ Y<sub>1031,4</sub> = .294 + .637 X<sub>1082</sub> Y<sub>1031</sub>,1 " .480 + .618 X<sub>1082</sub>  $x_{1031,2} = .389 + .635 x_{1082}$ Y<sub>1031</sub>,1 = .266 + .649 X<sub>1152</sub>  $Y_{1031,2} = .130 + .684 X_{1152}$  $Y_{1031,4} = .168 + .679 X_{1152}$ 

(0.953)

SE =.484 .

(0.944) (0.675) (965.0) (0.510)

SE = .654

(0.982)

\* represents R-Square

Table II.2



Figure II.5 : Errors of Estimation Methods for Station 125



Figure II.6 : Errors of Estimation Methods for Station 103-1

#### CHAPTER III

# ANALYSIS OF MONTHLY PRECIPITATION

When a time series is studied, the observed value of the series at a particular time period should be viewed as a random value. If the series exhibits a systematic change over time, some transformation procedures can be applied to the series to make it stationary. Detrending and deseasonalizing procedures were employed in this study.

#### III.1 Removal of Long Term Trend

The trend in a time series can be defined as any systematic change in the level with respect to time. When a time series is steadily increasing or decreasing over time, the annual or some periodic mean of the series also intends to change through the whole period. In this case, the analysis of the data might not yield consistent results because of the presence of such trend. Hence it is necessary to remove the trend over time if it exists.

To examine if the monthly precipitation has the over-year trend, the annual mean of each precipitation station is computed as

$$\overline{X}(y) = \frac{1}{12} \sum_{m=1}^{12} X(m, y) , \quad y = 1, 2, \dots, Y \quad (III.1)$$

in which X(y) is the arithematic average of monthly precipitation in year y, X(m,y) is the observed monthly precipitation in month m and year y, and Y is the total number of years in the record. The annual mean precipitations for all 21 stations are plotted in Figures III.1-III.7. Although there are some fluctuations in the annual average precipitations, all stations do not appear to have any visible over-year changes in the annual mean. There is a slightly large fluctuation for the first several years at station 109, but the annual mean seems to stabilize after that. The annual mean at station 121-A has a decreasing trend for the last three years, but there was no more data afterward because the station was removed.

A simple linear regression analysis is performed to fit the annual average precipitation over time. The resulting regression equations are shown in Table III.1 with the p-values for the slope. As can be seen, all the slopes of the fitted regression lines are insignificant at both 10% and 5% levels. Based on these results, the conclusion was made that there were no long-term trend in monthly precipitation. Therefore, it is not necessary to detrend the monthly precipitation data.





Precipitation Average Annual







Annual Average Precipitation







Annual Average Precipitation



Annual Average Precipitation

Station	Regression Equations for Annual Trend
101	$\bar{\mathbf{X}} = 2.22 - 0.0132  \mathrm{Y},  (0.815)^*$
102	= 2.11 + 0.0708 Y, (0.062)
103-1	= 3.90 - 0.1041 Y, (0.316)
103-2	= 3.68 - 0.0404 Y, (0.387)
103-A	= 2.64 + 0.0182 Y, (0.627)
106	= 1.96 + 0.0506 Y, (0.166)
108-2	= 3.33 + 0.0856 Y, (0.206)
108-A	= 3.71 - 0.0795 Y, (0.443)
109	= 3.55 + 0.1219 Y, (0.453)
115-2	= 3.32 + 0.0793 Y, (0.269)
115-A	= 3.76 - 0.0336 Y, (0.555)
119	= 2.24 + 0.0409 Y,  (0.409)
120	= 2.61 + 0.0370 Y, (0.562)
121	= 3.41 + 0.0002 Y, (0.997)
121-A	= 4.34 - 0.2414 Y,  (0.133)
122	= 3.05 + 0.0177 Y, (0.813)
123	= 2.93 + 0.0182 Y, (0.762)
124	= 2.96 + 0.0025 Y, (0.959)
125	= 2.17 + 0.1422  Y,  (0.130)
126	= 3.42 - 0.0869 Y, (0.135)
127	= 1.81 + 0.0845 Y, (0.211)

X : Annual mean of monthly precipitation
Y : number of years,
\* : P-value of regression coefficient

Table III.1 : Regression Equations of Annual Mean Precipitation

#### III.2 Removal of Periodicity

In monthly precipitation time series, some types of seasonal patterns would be likely to exist at 12-month intervals. This fluctuation reveals a seasonal cycle or periodicity that recurs about every 12 months. To make the time series stationary, deseasonalization is needed to remove the within-year seasonal fluctuation in the monthly precipitation time series. Two methods (Parzen, 1981) were used in this study to deseasonalize the monthly precipitation data.

# III.2.1 Using Monthly Sample Statistics

The monthly averages of the total precipitation for each station on a given month m can be computed as

$$\overline{X}(m) = \frac{1}{Y} \sum_{y=1}^{Y} X(m, y)$$
,  $m = 1, 2, ..., 12$  (III.2)

in which  $\overline{X}(y)$  is the average precipitation for month m. Then, the standardization procedure is applied to obtain the deseasonalized data as

$$Z(m, y) = \frac{X(m, y) - \overline{X}(m)}{S(m)}$$
(III.3)

in which S(m) is the sample standard deviation of monthly precipitation for month m,

$$S(m) = \sqrt{\frac{\sum_{y=1}^{Y} [X(m, y) - \overline{X}(m)]^2}{Y - 1}} , m = 1, 2, ..., 12$$
(III.4)

The sample statistics of the monthly precipitation are shown in Tables III.2 and III.3. The standardized monthly precipitations for stations 106 and 108-2 are presented in Figures III.8 and III.9 to compare with the original time series shown in Chapter I. As can be observed, the seasonal fluctuation that appeared in the original time series data is removed. The standardized monthly precipitation, then, can be considered stationary having zero mean and unit variance.

## III.2.2 Using Fourier Series

Another method of deseasonalization is to fit the monthly average mean by a Fourier series (Bloomfield, 1975). Then, the above standardization procedure can be performed using the fitted average, instead of
Station	Month					
	Jan.	Feb.	Mar.	Apr.	May.	Jun.
101	2.03	1.51	2.34	2.47	2.40	1.24
102	2.58	2.15	3.33	3.54	2.85	1.41
103-1	3.04	1.39	3.67	4.05	3.33	1.61
103-2	3.53	2.82	4.26	4.43	3.78	1.69
103-A	2.94	2.29	3.47	3.90	3.29	1.59
106	2.34	1.79	2.85	3.06	2.82	1.27
108-2	6.28	5.09	4.45	5.77	4.97	4.55
108-A	3.12	2.48	3.78	3.99	3.32	1.59
109	6.51	5.02	6.33	6.25	4.48	2.02
115-2	4.85	3.79	5.48	5.55	4.45	1.91
115-A	4.25	3.10	4.87	4.82	3.92	1.67
119	2.63	1.95	3.07	3.55	3.16	1.40
120	3.03	2.35	3.48	3.74	3.19	1.53
121	4.23	3.01	4.73	4.92	4.12	1.75
121-A	4.03	3.01	4.70	4.70	4.09	1.68
122	4.00	2.92	4.13	4.17	3.69	1.52
123	3.19	2.83	3.87	4.37	3.48	1.50
124	3.18	2.49	3.84	4.22	3.37	1.57
125	4.82	3.37	5.06	4.34	3.45	1.55
126	3.05	2.64	3.55	3.64	2.90	1.74
127	2.39	1.94	3.01	3.31	2.70	1.27

Table III.2 : Monthly Average Precipitations in Snowy Range Observatory

(	C	0	n	t	1	n	u	e	d	)	

Station	Month					
	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
101	2.50	1.52	1.47	1.63	2.29	2.49
102	2.51	1.48	1.86	2.18	3.31	3.26
103-1	2.71	1.85	1.82	2.47	3.83	3.56
103-2	2.70	1.90	1.90	2.61	4.19	3.87
103-A	2.61	1.75	1.84	2.29	3.41	3.21
106	2.49	1.56	1.63	1.95	2.69	2.79
108-2	2.27	2.92	2.19	2.32	3.29	5.00
108-A	2.81	2.00	1.96	2.47	3.28	3.62
109	3.02	2.23	2.39	3.23	5.06	5.81
115-2	2.83	2.18	2.09	3.31	4.78	5.47
115-A	2.70	1.96	1.87	2.75	4.01	5.03
119	2.46	1.58	1.52	2.15	2.97	2.84
120	2.58	1.64	1.63	2.24	3.58	3.55
121	2.87	1.97	1.91	2.78	4.46	4.56
121-A	2.57	1.93	1.89	2.76	4.34	4.44
122	2.77	2.00	2.09	2.75	3.63	4.17
123	2.51	1.77	1.77	2.63	3.82	3.72
124	2.49	1.74	1.77	2.53	3.48	3.61
125	2.53	1.86	1.80	2.76	4.25	4.76
126	2.70	2.26	1.94	2.53	3.21	2.85
127	3.30	1.65	1.64	2.11	3.28	2.82

Table III.2 : Monthly Average Precipitations in Snowy Range Observatory

Station	Month					
	Jan.	Feb.	Mar.	Apr.	May.	Jun.
101	1.32	1.18	0.72	1.33	0.94	0.84
102	1.84	1.35	1.33	1.95	1.49	0.88
103-1	2.04	1.61	1.46	2.28	1.46	1.05
103-2	2.28	1.75	1.62	2.58	1.66	1.07
103-A	2.46	1.38	1.24	2.07	1.46	1.06
106	1.57	1.31	1.36	1.61	1.13	0.90
108-2	4.46	3.55	1.96	2.18	1.71	1.74
108-A	1.94	1.35	1.36	1.69	1.39	1.04
109	4.52	3.43	3.24	2.92	1.65	0.99
115-2	3.07	2.20	1.94	2.58	1.75	1.34
115-A	2.48	1.47	2.01	2.23	1.52	1.02
119	1.81	1.33	1.25	1.91	1.46	0.92
120	2.18	1.87	1.63	2.29	1.50	0.98
121	2.67	1.55	1.72	2.47	1.78	0.94
121 <b>-</b> A	2.72	1.84	1.78	2.34	1.97	1.02
122	2.18	1.76	1.75	1.92	1.77	1.00
123	2.32	1.96	1.53	2.19	1.48	0.93
124	2.19	1.48	1.64	2.03	1.43	0.98
125	4.11	2.10	2.40	2.11	1.51	1.04
126	1.72	1.59	1.62	1.28	1.15	1.00
127	1.77	1.44	1.42	2.02	1.52	0.97

Table III.3 : Standard Error of Monthly Precipitation in Snowy Range Observatory

(continued)

Station	Month					
	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
101	1.50	0.65	0.94	0.68	1.46	1.63
102	1.83	0.75	1.14	0.81	1.92	1.86
103-1	1.61	0.93	1.05	0.87	2.44	2.44
103-2	1.56	0.97	1.14	0.89	2.51	2.42
103-A	1.53	1.03	0.94	0.68	1.99	1.84
106	1.72	0.78	0.97	0.71	1.52	1.57
108-2	1.43	1.30	0.96	1.43	1.04	2.75
108-A	1.42	0.90	1.05	0.59	1.76	2.13
109	1.63	0.94	1.23	1.40	2.25	3.24
115-2	1.74	1.02	1.18	1.04	2.65	3.28
115-A	1.65	0.90	0.99	0.79	2.43	2.94
119	1.46	0.78	1.12	0.71	2.00	1.67
120	1.57	0.86	1.08	0.89	2.34	2.30
121	1.70	0.89	1.17	0.91	2.42	2.56
121-A	1.82	0.97	1.19	0.89	2.28	2.66
122	1.47	0.95	1.15	0.72	1.74	2.50
123	1.40	0.93	1.18	0.87	2.32	2.32
124	1.36	1.81	1.04	0.76	2.06	2.12
125	1.45	0.98	2.11	0.69	2.12	2.40
126	1.41	1.22	0.92	0.63	1.17	1.61
127	3.10	0.99	1.13	0.95	2.23	1.83

Table III.3 : Standard Error of Monthly Precipitation in Snowy Range Observatory



Precipitation (Inches) at Station 1082

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Precipitation (Inches) at Station 106

sample means. The Fourier series

$$\overline{X}_{F}(m) = \overline{\overline{X}} + \sum_{f=1}^{6} \left[ \alpha_{f} \cos\left(\frac{2\pi fm}{12}\right) + \beta_{f} \sin\left(\frac{2\pi fm}{12}\right) \right]$$
(III.5)

is an approach to find the "hidden periodicity" in a time series in which  $\overline{X}_{F}(m)$  is the fitted sample mean by a Fourier series in month m,  $\overline{\overline{X}}$  is an overall mean that is computed by

$$\overline{\overline{X}} = \frac{1}{T} \sum_{y=1}^{Y} \sum_{m=1}^{12} X(m, y)$$
 (III.6)

in which T is the total months in the time series used and  $\alpha_f$  and  $\beta_f$  are Fourier coefficients that can be computed as

$$\alpha_{f} = \frac{1}{6} \sum_{m=1}^{12} \overline{X}(m) \cos\left(\frac{2\pi fm}{12}\right) , f = 1, 2, \dots, 6 \qquad (III.7)$$

$$\beta_{f} = \frac{1}{6} \sum_{m=1}^{6} \overline{X}(m) \sin\left(\frac{2\pi fm}{12}\right) , f = 1, 2, \dots, 6$$
 (III.8)

It should be noted that not all Fourier coefficients are statistically significant. To test their significance for each frequency f, a test statistic  $v_f$  (Parzen, 1981)

$$v_f = \frac{T \left(\alpha_f^2 + \beta_f^2\right)}{\overline{S}}$$
(III.9)

is used in which  $\overline{S}$  is the average of the monthly sample standard deviation of the monthly precipitation that can be computed by

$$S = \frac{1}{12} \sum_{m=1}^{12} S(m)$$
 (III.10)

The test statistic  $v_f$  has a  $\chi^2$  distribution with 2 degree of freedom. If the statistic  $v_f > 12.0$ , for f=1,2,...,5 and  $v_f > 3.84$  for f=6, the coefficients at the corresponding frequency can be accepted as significant and thus the cycles of the period 12/f, for f=1,2,...,6, which correspond to 12, 6, 4, 3, 2.4 and 2 months are also significant. Then the seasonal means can be estimated using those significant coefficients. The estimated coefficients are presented in Tables III.4 and III.5. These tables also show the results of significant tests of the Fourier coefficients. By the results of this analysis, the frequencies f=1, 2, 3 and 6 are mostly significant throughout all 21 stations. The corresponding coefficients are used to estimate the mean precipitation for each month. Using the results from Fourier analysis, the fitted monthly mean,  $\overline{X}_{\rm F}({\rm m})$ , is used to transform the detrended time series into a monthly stationary series

$$Z_F(m, y) = \frac{X(m, y) - \overline{X}_F(m)}{S(m)}$$
(III.11)

Station			α	٤		
	f=1	f=2	f=3	f=4	f=5	f=6
101	0.07	0.24	20	06	0.19	0.36
102	03	0.43	31	09	0.11	0.40
103-1	01	0.52	34	01	0.10	0.41
103-2	0.01	0.56	32	0.03	0.11	0.51
103-A	0.09	0.45	24	03	0.09	0.42
106	0.08	0.34	21	06	0.19	0.40
108-2	0.57	0.25	0.57	02	30	36
108-A	0.06	0.35	25	10	0.21	0.35
109	17	0.30	17	· <b></b> 17	0.05	0.54
115-2	13	0.62	28	09	0.25	0.38
115-A	08	0.52	26	16	0.29	0.38
119	0.14	0.45	20	03	0.15	0.39
120	02	0.49	30	02	0.13	0.41
121	03	0.62	30	05	0.16	0.56
121-A	05	0.61	27	00	0.19	0.52
122	10	0.37	12	09	0.25	0.47
123	01	0.52	29	0.02	0.13	0.31
124	0.05	0.50	25	09	0.15	0.33
125	29	0.34	29	15	0.18	0.54
126	0.05	0.16	26	0.30	0.05	0.28
127	0.04	0.33	47	06	0.14	0.54

Table III.4 : Estimates of Fourier Coefficients,  $\alpha_f$ 

Station			β	¢	,	
	f=1	f=2	f=3	ff=4	f=5	f=6
101	23	0.05	35*	16	12	00*
102	60*	17*	42*	14	16	00*
103-1	70*	16*	50*	23	07	00*
103-2	95*	20*	51*	22	02	00*
103-A	64*	17*	45*	23	08	00*
106	41*	09*	40*	19	10	00*
108-2	-1.60*	0.49*	29*	07	22	00*
108-A	68*	15*	51*	21	07	00*
109	-2.10*	12	67*	36	0.04	00*
115-2	-1.53*	20*	70*	23	0.03	00*
115-A	-1.30*	19*	72*	16	0.02	00*
119	55*	14*	41*	28	05	00*
120	74*	06*	47*	20	08	00*
121	-1.19*	13*	65*	27	0.02	00*
121-A	-1.19*	19*	60*	20	0.04	00*
122	96*	10*	52*	26	02	00*
123	91*	258	48*	27	11	00*
124	84*	22*	50*	23	04	00*
125	15*	04	55*	19	0.15	00*
126	53*	19	31*	27	0.03	00*
127	35	04	41*	34	21	00*

Table III.5 : Estimates of Fourier Coefficients,  $\beta_f$  (\* represents  $v_f$  is significant)

#### CHAPTER IV

## ANALYSIS OF SPATIALLY NONSTATIONARY MONTHLY PRECIPITATION

In Chapter III, the monthly total precipitation data under study shows the nonstationarity over the study area (see Table III.2 and III.3) indicated by that the mean and the variance are not constant. It is, therefore, necessary to have a mechanism to estimate the mean as well as the variance in space so that the standardized stationary random field can be transformed back to preserve the original spatial nonstationary characteristics. Firstly, two different methods that do not consider spatial correlation were used to estimate the monthly average precipitation over the space. The inverse distance weighing technique and the Gaussian smoothing method were used. Secondly, to consider the spatial correlation between stations at Snowy Range Observatory, the nonstationary kriging technique was used to estimate the monthly total precipitation over the space.

## IV.1 Estimation without Considering Spatial Correlation

The inverse distance weighing technique and the Gaussian smoothing weighing method use a weighted average of the values from the recording stations to estimate the monthly average precipitation at the specified ungaged location. The weights computed by inverse distance or Gaussian smoothing techniques are only functions of the physical distance between two stations. They do not explicitly account for the spatial variability of monthly precipitation under study. These two techniques also do not allow for computation of the reliability of the estimates.

#### IV.1.1 Inverse Distance Method

The linear inverse distance weighing technique has the same concept as the IDLIN technique in Chapter II used for estimating the missing values in which the contributing weight of the existing station to ungaged location is determined as

$$w_{s,IV} = \frac{\left(\frac{1}{D_s}\right)}{\sum_{i=1}^{S} \left(\frac{1}{D_i}\right)}$$
(IV.1)

where  $D_s$  is the distance between the specified ungaged location and a recording station s, S is the total number of stations used in estimation, and  $w_{s,IV}$  is the weight for the station s by inverse distance method. Then, the monthly average precipitation at the ungaged location,  $\overline{X}_{0,IV}(m)$ , for month m, is estimated by

$$\overline{X}_{0,IV}(m) = \sum_{s=1}^{S} w_{s,IV} \ \overline{X}_{s}(m)$$
(IV.2)

where  $\bar{X}_s(m)$  is the calculated total monthly average precipitation at the recording station s in a given month m. The method does not consider any other factors affecting the estimate except the distances between two locations. This technique also can be employed to estimate the variance of monthly precipitation at the ungaged location in a similar manner as

$$\hat{S}_{0,IV}^{2}(m) = \sum_{s=1}^{S} w_{s,IV} S_{s}^{2}(m)$$
 (IV.3)

in which  $S_{0,IV}^2(m)$  and  $S_s^2(m)$  are the variances at the ungaged location and the recording station s, respectively.

## IV.1.2 Gaussian Smoothing Method

The Gaussian smoothing technique (Borgman, 1990; Kallianpul, et al., 1988) uses the Gaussian function

$$f(D_s) = e^{-\frac{1}{2Q}D_s^2}$$
 (IV.4)

to compute the weights where  ${\tt Q}$  is a constant representing the effective width of smoothing. The weights are determined by

$$w_{s,GS} = \frac{f(D_s)}{\sum_{i=1}^{S} f(D_i)} , s = 1, 2, \dots, S$$
(IV.5)

The value of Q can be determined subjectively with the idea that the larger the value of Q is, the more smoothing the result will be. A very small value of Q allows only those recording stations in close vicinity of the estimation point to be used to estimate the monthly precipitation at the ungaged location. As can be seen, the weight also depends only on the distance. The estimates of monthly average precipitation can be determined by

$$\overline{X}_{0,GS}(m) = \sum_{s=1}^{S} w_{s,GS} \ \overline{X}_{s}(m)$$
 (IV.6)

In this study, Fibonacci search technique (Luenberger, 1984) is applied to find the optimal Q which minimizes the error associated with the estimates using the observed data at all recording stations. The objective function is to minimize

$$\varepsilon^{2}(m|Q) = \sum_{s=1}^{S} \left[ \overline{X}_{s}(m) - \overline{X}_{s,GS}(m|Q) \right]^{2} \qquad (IV.7)$$

in which  $\overline{X}_{s,GS}(m)$  is the estimates of monthly average precipitation resulting from Gaussian smoothing weights. This process can also be applied to estimate the variance,  $S_s^2(m)$ . The optimal Q's for estimating  $\overline{X}_s(m)$  and  $S_s^2(m)$  for each month, m=1,2,...,12, by the Fibonacci search are shown in Table IV.1.

## IV.2 Estimation considering Spatial Correlation (Kriging)

Unlike the inverse distance weighing technique and Gaussian smoothing weighing method, a geostatistical kriging method is based on the structure of the spatial variability of monthly total precipitation (Royle et al., 1980; Borgman, 1990; Bras et al., 1985). The measured monthly total precipitation in Snowy Range watershed allows one to analyze the spatial correlation structure in the study area and to incorporate such information to determine the optimum weight which minimizes the variance estimation. The kriging procedure has been developed around the variogram or covariance function and the best linear unbiased estimation.

# IV.2.1 Analysis of Spatial Correlation Structure

The computation of the correlation matrix is essential in kriging to find the variogram or covariance structure. Computation of the spatial correlation structure requires that the random field is stationary. Therefore, the standardized monthly total precipitation series described in Chapter III were used. Since the standardization requires estimating the means and variances of the monthly total precipitation, the above Gaussian smoothing method and two other techniques were used. One technique was simply to use sample means and sample variances at the recording stations and the other used the monthly total precipitation statistics calculated by the Fourier Series described in Chapter III.

Using the standardized monthly total precipitation series, the functional relationship between correlations and distances computed by the three different methods are plotted in Figures IV.1, IV.2 and IV.3. -All three plots are similar thus indicating that the correlation-distance relationship is not sensitive to the method used to compute the local mean and variance of the monthly total precipitation in Snowy Range watershed. It is interesting to observe that the correlation function is separated into two distinct parts. One, which locates at the lower part, parallel to

Month	Opti	Optimal Q				
	For Mean	For Variance				
January	0.3819663	0.5280327				
February	0.3068254	0.2938475				
March	0.2755167	0.7782433				
April	0.3068254	0.4530374				
May	0.5197246	0.1029390				
June	0.5322481	0.3942531				
July	0.2003758	0.8325485				
August	0.3318724	0.5290438				
September	0.2755167	0.5932542				
October	0.1565436	0.2842049				
November	0.2191610	0.3290381				
December	0.4383220	0.2374398				

Table IV.1 : Optimal Q for Gaussian Smoothing





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# Monthly Precipitation

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the horizontal axis indicating that the correlation associated these points is a constant against the distances. This part consists of 20 points and all these points are related to station 108-2. This implies that the monthly precipitation collected by station 108-2 is independent of the catch by all other stations. This lower part associated with station 108-2 is assumed to be a constant having the correlation value of the average of the total 20 points.

The upper part of the plot reveals an inverse linear relationship with the distances. This means that the closer the distance between two stations is, the higher the correlation is. These correlations were fitted by regression to find the structure for the three different methods. The resulting regression equations that define the functional relationship for correlation between station s and s',  $\rho(d_{s,s'})$  is as follows: when station 108-2 is considered to compute correlation, for all three techniques

$$\hat{\rho}(d_{s\,s'}) = constant \qquad (IV.8)$$

When all other stations are used to calculate the correlation, the resulting regression equation, using sample means and variances, is

$$\hat{\rho}(d_{s,s'}) = 0.928 - 0.0263 d_{s,s'}$$
,  $R^2 = 36.6\%$  (IV.9)

When using the Gaussian smoothing method the regression equation is

$$\hat{\rho}(d_{s,s'}) = 0.914 - 0.0257 d_{s,s'}$$
,  $R^2 = 30.0\%$  (IV.10)

and using the Fourier Series it is

$$\hat{\rho}(d_{s,s'}) = 0.901 - 0.0291 d_{s,s'}$$
,  $R^2 = 38.4\%$  (IV.11)

The resulting regression lines from the three different techniques are not distinguishable. Hence the correlation relation computed by the sample means and the sample variances is adapted in the further study. It should also be noted that there exists a nugget (random) effect (indicated in Figure IV.1) in these plots. The correlation function used in the nonstationary kriging for this study is

$$\hat{\rho}(d_{s,s'}) = \begin{cases} 0.028, d_{s,s'} > 0, & (IV.12) \\ & \text{for } s' = \text{station } 108-2 \text{ and} \\ 0.928 - 0.0263 d_{s,s'}, d_{s,s'} > 0, & (IV.13) \\ & \text{for } s' \neq \text{station } 108-2 \\ 1, d_{s,s'} = 0 & \text{for all } s, s' & (IV.14) \end{cases}$$

From the above correlation function, the covariance function can be computed as

$$C(d_{s,s'}) = C(s,s')$$
  
=  $\sigma(s)\sigma(s')\rho(d_{s,s'})$  (IV.15)

where  $C(d_{s,s'})$  and  $\rho(d_{s,s'})$  are the covariance and the correlation between two stations s and s' separated by distance  $d_{s,s'}$ , respectively.

## IV.2.2 Nonstationary Kriging

Nonstationary kriging proposes that the monthly total precipitation value over the specified block or point can be estimated by the known functions,  $X_{m,s}$  for s=1,2,...,S, as

$$\hat{X}_{m,s_0} = \sum_{s=1}^{S} w_s X_{m,s}$$
 (IV.16)

where S is the total number of recording precipitation stations in the region. The basic function,  $X_{m,s}$ , is known, but the weighing coefficients,  $w_s$ , are to be determined.

#### IV.2.2.1 Definition

The mean of the time-space random process,  $X_{\tt m,s},$  which is assumed to be second-order and nonstationary, is defined as

$$\boldsymbol{\mu}_{m,s} = \mathbf{E}\left[X_{m,s}\right] \tag{IV.17}$$

where s represents a location in a region and m is a given month (Borgman, 1990). The variance can also be expressed as

$$\sigma_{m,s}^{2} = \mathbb{E}\left[\left(X_{m,s} - \mu_{m,s}\right)^{2}\right]$$
 (IV.18)

The covariance allows modeling of the spatial dependence of the random process  $X_{\rm m,s}.$  The covariance function at any two locations, s and s' for month m, is defined as

$$C_{m}(s, s') = \mathbf{E}[(X_{m,s} - \mu_{m,s})^{T}(X_{m,s'} - \mu_{m,s'})] \quad (IV.19)$$

where  $s=1,2,\ldots,S$  and  $s'=1,2,\ldots,S$ . If s=s',  $C_m(s,s')$  is called autocovariance and if not,  $C_m(s,s')$  is called cross-covariance.

The objective of nonstationary kriging is to estimate the monthly total precipitation over the specified area B in a given month m as

$$\hat{X}_{Bm} = \frac{1}{B} \int_{B} X_{m,s} \, ds \quad , m = 1, 2, \dots, 12 \tag{IV.20}$$

When the block B reduces to a point, then

$$\hat{X}_{Bm} = X_{m,s_0} \tag{IV.21}$$

To construct the contour maps of the mean and the error, the use of nonstationary point estimation is preferred. The expectation of point kriging is

$$\mu_{m,s_0} = \mathbb{E}[X_{m,s_0}] , m=1,2,\dots,12$$
 (IV.22)

IV.2.2.2 Formulation of Nonstationary Kriging Model

The point monthly total precipitation in month m at the estimation point  $s_0$  can be expressed as a linear combination of the observed data,  $X_{\rm m,s},$  for s=1,2,...,S, by

$$\hat{X}_{m,s_0} = \sum_{s=1}^{S} w_s X_{m,s}$$
 (IV.23)

The mean square error of monthly total precipitation at a given estimation point  $s_0$ , in a given month m, can be rewritten as

$$\sigma_{E}^{2}(m) = \mathbf{E}\left[\left(\hat{X}_{m,s_{0}}^{*} - X_{m,s_{0}}^{*}\right)^{2}\right] = \mathbf{E}\left[\left(\sum_{s=1}^{S} w_{s}X_{m,s}^{*} - X_{m,s_{0}}^{*}\right)^{2}\right]$$
$$= \mathbf{E}\left[\left(\sum_{s=1}^{S} w_{s}(X_{m,s}^{*} - \mu_{m,s}) + \sum_{s=1}^{S} w_{s}\mu_{m,s}^{*} - (X_{m,s_{0}}^{*} - \mu_{m,s_{0}}^{*}) - \mu_{m,s_{0}}^{*}\right)^{2}\right]$$

Assume that the estimator is unbiased, that is,

$$\mathbf{E}\left[\hat{X}_{m,s_{0}} - X_{m,s_{0}}\right] = 0 \tag{IV.25}$$

Then the mean square error,  $\sigma_{\rm E}^{2}({\rm m})$ , is reduced to

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$$= \mathbf{E}\left[\left(\sum_{s=1}^{S} w_{s}(X_{m,s} - \mu_{m,s}) - (X_{m,s_{0}} - \mu_{m,s_{0}}) + (\sum_{s=1}^{S} w_{s}\mu_{m,s} - \mu_{m,s_{0}})\right)^{2}\right]$$
  
$$= \mathbf{E}\left[\left(\sum_{s=1}^{S} w_{s}(X_{m,s} - \mu_{m,s})\right)^{2}\right] + \mathbf{E}\left[(X_{m,s_{0}} - \mu_{m,s_{0}})^{2}\right]$$
  
$$-2\mathbf{E}\left[\sum_{s=1}^{S} w_{s}(X_{m,s} - \mu_{m,s}) (X_{m,s_{0}} - \mu_{m,s_{0}})\right]$$

$$\sigma_{E}^{2}(m) = \sum_{s=1}^{S} \sum_{s'=1}^{S} w_{s} w_{s'} C_{m}(s, s') + C_{m}(s_{0}, s_{0}) - 2 \sum_{s=1}^{S} w_{s} C_{m}(s, s_{0})$$
$$= \mathbf{w}^{T} C_{m} \mathbf{w} + \sigma_{m, s_{0}}^{2} - 2 \mathbf{w}^{T} C_{m, s_{0}}$$

The elements on the right-hand side of Eq.(IV.26) can be computed, using information extracted from the data, as

$$C_{m}(s, s') = S_{m,s} S_{m,s'} \hat{\rho}_{m}(d_{s,s'}) , \qquad (IV.27)$$

$$\hat{\sigma}_{m,s_0}^2 = \hat{S}_{m,s_0}^2 = \sum_{s=1}^{S} w_{s,IV} S_{m,s}^2$$
, (IV.28)

$$C_m(s, s_0) = s_{m,s} \hat{s}_{m,s_0} \hat{\rho}_m(d_{s,s_0}) , \qquad (IV.29)$$

$$\mu_{m,s_0} = \sum_{s=1}^{S} w_{s,IV} \ \mu_{m,s} \tag{IV.30}$$

in which  $\rho_{\rm m}({\rm s},{\rm s}')$  and  $\rho_{\rm m}({\rm s},{\rm s}_0)$  are the correlation estimated by the structured correlation function, Eq.(IV.12), (IV.13) or (IV.14), between stations s and s' and between ungaged point  ${\rm s}_0$  and station s in month m, respectively;  ${\rm w}_{\rm s,IV}$  is the inverse distance weight contributed from the existing station s;  $\mu_{\rm m,s}$ ,  ${\rm s}_{\rm m,s}$  and  $\sigma_{\rm m,s}^2$  are the mean, standard deviation, and variance measured at station s in month m, respectively.

After the above terms are computed, the nonstationary kriging model can be expressed as the following

(Model I)

Minimize

$$\boldsymbol{\sigma}_{E}^{2}(\boldsymbol{m}) = \boldsymbol{w}^{T} \boldsymbol{\mathbb{C}}_{m} \boldsymbol{w} + \boldsymbol{\sigma}_{m, s_{0}}^{2} - 2 \boldsymbol{w}^{T} \boldsymbol{\mathcal{C}}_{m, s_{0}} \qquad (\text{IV.31a})$$

subject to

 $(\text{IV.31b}) \quad \boldsymbol{w}^{T}\boldsymbol{\mu}_{m} = \boldsymbol{\mu}_{m,s_{0}} ,$ 

$$\boldsymbol{w}^T \mathbf{1} = 1 \quad , \tag{IV.31c}$$

 $W_s \ge 0$  , s=1, 2, ..., S (IV.31d)

The commonly used kriging model involves only one constraint Eq.(IV.31b). The solution to such model do not ensure that all weighing factors are nonnegative and add up to one. Therefore, constraint Eqs.(IV.31c) and (IV.31d) are added to the kriging model.

In model I, the local variance,  $\sigma_{m,s}^2$ , is considered as a constant. However, it is actually a function of the variances measured at every existing station,  $\sigma_{m,s}^2$ 's, and the corresponding weights which are unknown. Therefore,  $\sigma_{m,s}^2$  should be considered as a decision variable and model I is modified as the following

(Model II)

Minimize

 $\boldsymbol{\sigma}_{E}^{2}(\boldsymbol{m}) = \boldsymbol{w}^{T} \boldsymbol{\mathbb{C}}_{\boldsymbol{m}} \boldsymbol{w} + \boldsymbol{\sigma}_{\boldsymbol{m}, s_{0}}^{2} - 2 \boldsymbol{w}^{T} \boldsymbol{c}_{\boldsymbol{m}, s_{0}}$ 

$$= \mathbf{w}^{T} \mathbb{C}_{m} \mathbf{w} + \sigma_{m,s_{0}}^{2} - 2\mathbf{w}^{T} [\sigma_{m,s_{0}} s_{m,s} \hat{\rho}_{m} (d_{s,s_{0}})]$$
(IV.32a)

subject to

$$\boldsymbol{w}^{T}\boldsymbol{\mu}_{m} = \boldsymbol{\mu}_{m,s_{0}} \quad , \tag{IV.32b}$$

$$\boldsymbol{w}^T \boldsymbol{1} = 1 \quad , \qquad (IV.32d)$$

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$$\boldsymbol{w}^{T}\boldsymbol{\sigma}_{m}^{2} = \boldsymbol{\sigma}_{m,s_{0}}^{2} , \qquad (\text{IV.32c})$$

$$w_s \ge 0$$
 ,  $s = 1, 2, \dots, S$  (IV.32e)

in which  $\sigma_{m,s}^2$  is an additional decision variable representing unknown variance at the estimation point  $s_0$ .

## IV.2.2.3 Solution Procedure

The Lagrange multiplier method has been used to solve the conventional kriging model subject to only a single unbiased constraint (Borgman, 1990; Journel et al., 1978). However, the Lagrange multiplier method is inefficient for solving models I and II formulated above, which involves several constraints, including non-negativity constraints. In this study, a nonlinear programming code, called GRG2 (Lasdon et al., 1986), is used which is based on the generalized reduced gradient algorithm (Lasdon, 1979; Murtagh, 1978)

## IV.3 Comparison Study

In this section, the results of contour maps for means and errors obtained by the different methods for each month are compared.

# IV.3.1 <u>Comparison between Nonstationary Kriging Models I</u> and II

Figure IV.4 shows the mean precipitation contour maps for January derived by nonstationary kriging models I and II. It appears that the two nonstationary kriging models do not yield a significant difference between the mean monthly precipitation with regard to its spatial distribution. The contour maps (see Figure IV.5) for the differences in means are almost flat at zero level. Slightly greater value of the mean precipitations are estimated by model II around stations 109 and 108-2, which are located in the lower-left corner of the study region. Model I, however, has the larger error than model II for every month except August (see Figure IV.6). The error patterns between the two models are distinctly different at the lower-center area (see Figure IV.5) or at the lower-left area (Appendix A). The contour maps for the remaining months are shown in Appendix A.



Figure IV.4 Comparison of Mean Contours from Model I & II for January



Figure IV.5 Contours of Difference between Model I & II for January



Figure IV.6 Comparison of Error Contours from Model I & II for January

# IV.3.2 <u>Comparison of Kriging Models, Inverse Distance and</u> <u>Gaussian Smoothing</u>

It is interesting to compare the contour maps resulting from the Gaussian smoothing weighing method, the inverse distance weighing technique and the kriging method. The contour for the means and the errors estimated by the Gaussian smoothing and the inverse distance methods for January are drawn in Figures IV.7 and IV.8, respectively. The mean precipitation pattern computed by the inverse distance method is quite similar to that computed by the two nonstationary kriging methods. However, the error estimated by the inverse distance technique is always greater over the entire study region than the error estimated by kriging procedures.

The means and the errors obtained by the Gaussian smoothing method vary greatly over the basin compared to those by the other two methods. The Gaussian smoothing technique yields larger values of monthly precipitation at the left area of the watershed than the others and smaller values in the right part.

## IV.3.3 <u>Discussion</u>

Models I and II yield almost identical spatial patterns of mean monthly total precipitation, but model I has a larger error in estimation. The reason for model II having smaller error is probably due to the fact that model II treats local variance as a decision variable. In model I, the local variance is considered as a constant computed by the initial weights,  $w_{s,IV}$ . The nonstationary kriging model I does not allow the local variance to use updated optimum weights.

The contour maps for the mean monthly total precipitation obtained by the inverse distance method have a similar pattern to those obtained by the nonstationary kriging. It, however, has larger errors than the kriging results (see Figure IV.7). This might be because the kriging is an optimization procedure which minimizes the estimation error while the inverse distance method does not.

The Gaussian smoothing method yields larger mean monthly total precipitations and errors than all other methods at the left portion of the study area and the lowest values at the right (see Figure IV.8). Referring to Table IV.1, the optimal Q's representing the effective width of smoothing, for the means and the variances have relatively small values less than 1. However, a lot of smoothing is done for estimating the monthly precipitations and the errors (see Figure IV.8).

This comparison study indicates that the kriging method is superior which could be attributed to the fact that the kriging procedure considers the spatial variability while the Gaussian smoothing and the inverse distance methods do not.



Figure 1V.7 Mean & Error Contours from Inverse Distance for January



Figure IV.8 Mean & Error Contours from Gaussian Smoothing for January

#### CHAPTER V

## PRECIPITATION NETWORK REDUCTION AND DESIGN

Operation and maintenance of large number of gage station in a remote area requires significant fiscal and manpower commitment. Currently, the Wyoming Water Research Center is contemplating the possibility of future budget reallocation. The task then is set out to examine the possible reduction of network size without sacrificing too much hydrologic information content in the Snowy Range Observatory.

In this Chapter, the formulation of the precipitation network design model that takes into account the spatial characteristics of monthly total precipitation in the study area was considered.

First, the subjective selection of the reduced precipitation network was made based on the geographical locations of the existing stations and other considerations such as the accessibility of the station and the aesthetic aspect. Since precipitation catches are correlated spatially, those stations having similar statistical characteristics for the monthly total precipitation can be reduced without affecting the hydrologic information contents in the study area. Consultations were made with the WRRC researchers and two reduced precipitation networks in the Snowy Range watershed were obtained. Of course, this reduced network so subjectively determined does not guarantee to be the optimum result in statistical information contents.

The model for optimal precipitation network configuration is also considered. The objective of the model is to determine the optimal precipitation network configuration in the Snowy Range watershed that optimizes some types of statistical information measures. In particular, the nonstationary kriging variance is used to construct the object function of the model. In hydrologic network design, there are, generally, two criteria used in the model: (1) accuracy and (2) cost. Some papers that treated this subject considered both (Bras et al., 1976; Loaiciga, 1989) or only accuracy (Shamsi et al., 1988; Bardsley, 1985; Sorman, 1983). This study shall only consider accuracy to find the minimum kriging variance. Mixed integer programming (MIP) is applied to this problem.

## V.1 Network Reduction

Two reduced network configurations were considered (see Figures V.1 and V.2). One consisted of 15 stations (101, 102, 103-1, 103-2, 106, 108-2, 109, 115-2, 119, 120, 121, 122, 123, 124 and 127) and the other had 12 stations (101, 103-2, 106, 108-2, 109, 115-2, 119, 120, 121, 122, 123 and 124). The contour maps for the means and the variances of the monthly total precipitation were constructed for each month. The resulting contour maps based on the reduced networks with 15 and 12 stations were compared with those of all 21 precipitation station.

Figure V.3 contains the error maps generated from the nonstationary kriging model I based on 15 and 12 stations, respectively. As can be seen, the errors increase as the number of retained stations decreases. The contour maps of the percentage increment in error associated with the reduced networks related to the full network of 21 stations are plotted in Figure V.4 for January representing the months (from October to April) during which the precipitation is most likely in the form of snow. Figure V.5 is for July representing the months (from May to September) during which the precipitation in general is in the form of rain. Contour maps of error percentage increment for the remaining months are shown in Appendix B. It was observed that, during June - September, the percentage increase in error associated with the reduced network is rather small. However, during the snow months (October-April), the error increases up to about 27% in the upper part of the watershed with a reduced network of 15 Further reduce the network to 12 stations resulted in a stations. slightly higher percentage increment in error to about 30%. The error increment associated with the reduced network in the lower part of the watershed is insignificant all year around. This comparison is also made using the nonstationary kriging model II (see Figures V.6 and V.7). The results from model II are similar to those from model I.

The comparison is also performed for the mean monthly precipitation (see Figure V.8). The results show that the differences in means between 15 and 21 or 12 and 21 stations are not distinctly different.

## V.2 Optimal Network Design

#### V.2.1 <u>Model</u>

Consider that there are K estimation points and S existing precipitation stations in the study area. The objective is to identify the optimal subset of existing stations to be retained to minimize the estimation error. Based on Eq.(IV.31a), the error, measured by the MSE, at the k-th estimation point within the study area for the month m is

$$\hat{\boldsymbol{\sigma}}_{k}^{2}(\boldsymbol{m}) = \boldsymbol{w}_{k}^{T} \boldsymbol{\mathbb{C}}_{m} \boldsymbol{w}_{k} - 2 \boldsymbol{w}_{k}^{T} \boldsymbol{c}_{m,k} + \boldsymbol{\sigma}_{m,k}^{2} \qquad (V.1)$$



Figure V.1 : Contour Map of Error for Reduced Network (Model I)



Figure V.2 : Contour Map of Percentage Difference in Error by Reduced Network ( Model 1)







Figure V.4 : Contour Map of Percentage Difference in Error by Reduced Network (Model II)



Figure V.5 : Comparison of Contour Map of Mean for Reduced Network by Model II
where  $\mathbf{w}_{k}$  is a Sxl vector of weights for the k-th estimation point,  $\mathbf{c}_{\mathrm{m},k}$  is a Sxl vector of the estimated covariance between the estimation point k and the existing stations S,  $\sigma_{\mathrm{m},k}^2$  represents the local variance at the estimation point k for month m estimated by the contributing weights and the corresponding variances at the retained stations. A representative measure of the estimation error for a study area is the aerial averaged error or, equivalently, the total aerial error

$$\sum_{k=1}^{K} \sigma_{k}^{2}(m) = \sum_{k=1}^{K} \left[ \boldsymbol{w}_{k}^{T} \mathbb{C}_{m} \boldsymbol{w}_{k} - 2 \boldsymbol{w}_{k}^{T} \boldsymbol{c}_{m,k} + \sigma_{m,k}^{2} \right]$$
(V.2)

In the network design context, the status of each existing station, to be retained or removed, is unknown. Therefore, zero-one integer variables,  $z_s$ , for s=1,2,...,S, are introduced to the model. If the station s is to be retained,  $z_s$  has the value of one, otherwise,  $z_s=0$ .

The objective of this precipitation network design problem is to minimize the total MSE over the specified area. The proposed mixed integer programming (MIP) model to optimally select the existing precipitation stations for month m is the following:

Minimize 
$$\sum_{k=1}^{K} \sigma_{k}^{2}(m) = \sum_{k=1}^{K} [\mathbf{w}_{k}^{T} \mathbb{C}_{m} \mathbf{w}_{k} - 2 \mathbf{w}_{k}^{T} \mathbf{c}_{m,k} + \sigma_{m,k}^{2}]$$
 (V.3)

The constraints in the model include the following:

(a) The sum of contributing weight from the retained stations to individual estimation point is unity.

$$w_k^T \mathbf{1} = 1$$
,  $k = 1, 2, \dots, K$  (V.4)

(b) The mean monthly total precipitation at the estimation point k is a linear weighted average of the monthly mean precipitation measured from the retained stations

$$\mathbf{w}_{k}^{T} \boldsymbol{\mu}_{m} = \boldsymbol{\mu}_{m,k}$$
,  $k = 1, 2, \dots, K$  (V.5)

(c) Because any existing station s, if retained, can possibly contribute to compute monthly precipitation at all K estimation points, the upper bound value for the sum of contributing weights from the existing station s will be K. If not retained  $(z_s=0)$ , the contribution from station s will be zero.

$$\sum_{k=1}^{K} w_{s,k} - z_s K \le 0 \quad , \quad s = 1, 2, \dots, S$$
 (V.6)

(d) The number of stations to be retained cannot exceed the specified number N determined by fiscal or geographical consideration.

$$\sum_{s=1}^{S} Z_s \leq N \quad , \tag{V.7}$$

(e) The contributing weights are non-negative.

 $w_k \ge 0$ ,  $k = 1, 2, \dots, K$ ,  $z_s = 0$  or 1,  $s = 1, 2, \dots, S$  (V.8)

The first and second constraints are the same as those of model I for nonstationary kriging described in Chapter IV.

The proposed optimal network design model is nonlinear. For each month, the MIP problem includes the zero-one integer variables,  $z_s$ , and non-negative real decision variables,  $w_k$ ,  $k=1,2,\ldots,K$ . The number of decision variables is S+SxK real-valued decision variables for the weighing factors and S zero-one integer decision variables. The number of constraints is 2K+S+1 (not including nonnegativity constraints).

## V.2.2 <u>Solution Algorithm</u>

Since the optimal network design model is a nonlinear MIP problem, there is no solution software immediately available. It is proposed to linearize the objective function by retaining the first order Taylor expansion term. The first-order Taylor expansion of the MSE for the given estimation point k about  $\mathbf{w}_k = \mathbf{w}_k^0$  is

$$f(\boldsymbol{w}_{k}) = \boldsymbol{w}_{k}^{T} \boldsymbol{\mathbb{C}}_{m} \boldsymbol{w}_{k} - 2 \boldsymbol{w}_{k}^{T} \boldsymbol{\sigma}_{m,k} + \sigma_{m,k}^{2}$$
(V.9)

$$= f(\boldsymbol{w}_{k}^{0}) + \nabla^{T} f(\boldsymbol{w}_{k}^{0}) (\boldsymbol{w}_{k} - \boldsymbol{w}_{k}^{0}) + \varepsilon \qquad (\forall.10)$$

$$= [f(\boldsymbol{w}_{k}^{0}) - 2 \boldsymbol{w}_{k}^{0} \mathbb{C}_{m} \boldsymbol{w}_{k}^{0} + \sigma_{m,k}^{2}(m)] + 2 \boldsymbol{w}_{k}^{T} \mathbb{C}_{m} \boldsymbol{w}_{k}^{0} - 2 \boldsymbol{w}_{k}^{T} \boldsymbol{c}_{m,k}$$
(V.11)

$$= constant_{k}^{0} + 2 \left[ \boldsymbol{w}_{k}^{T} \boldsymbol{C}_{m} \boldsymbol{w}_{k}^{0} - \boldsymbol{w}_{k}^{T} \boldsymbol{c}_{m,k} \right] \qquad (V.12)$$

$$= constant_{k}^{0} + 2 \boldsymbol{w}_{k}^{T} [\boldsymbol{\mathbb{C}}_{m} \boldsymbol{w}_{k}^{0} - \boldsymbol{c}_{m,k}] \qquad (V.13)$$

Substituting Eq.(V.13) into Eq.(V.3), the linearized objective function of the network design model is

Minimize

$$\sum_{k=1}^{K} f(\mathbf{w}_{k}) = \sum_{k=1}^{K} \left[ \mathbf{w}_{k}^{T} \mathbf{C}_{m} \mathbf{w}_{k} - 2 \mathbf{w}_{k}^{T} \mathbf{c}_{m,k} + \sigma_{m,k}^{2} \right]$$
$$\sim \sum_{k=1}^{K} constant_{k}^{0} + 2 \sum_{k=1}^{K} \mathbf{w}_{k}^{T} \left[ \mathbf{C}_{m} \mathbf{w}_{k}^{0} - \mathbf{c}_{m,k} \right] \qquad (V.14)$$

The above objective function is equivalent to

minimize

$$\sum_{k=1}^{K} \boldsymbol{w}_{k}^{T} \left[ \mathbb{C}_{m} \boldsymbol{w}_{k}^{0} - \boldsymbol{c}_{m,k} \right]$$
(V.15)

subject to

$$\mathbf{w}_{k}^{T} \mathbf{1} = 1$$
 ,  $k = 1, 2, \dots, K$  (V.16)

$$\mathbf{w}_{k}^{T} \boldsymbol{\mu}_{m} = \boldsymbol{\mu}_{m,k}$$
,  $k = 1, 2, \dots, K$  (V.17)

$$\sum_{k=1}^{K} w_{sk} - z_s K \le 0 \quad , \quad s = 1, 2, \dots, S$$
 (V.18)

$$\sum_{s=1}^{S} Z_s \leq N \quad , \tag{V.19}$$

$$w_k \ge 0$$
,  $k = 1, 2, \dots, K$ ,  
 $z_s = 0$  or 1,  $s = 1, 2, \dots, S$  (V.20)

The linearized network design model is then solved by the computer program ZOOM (Zero-One Optimization Methods) developed by Marsten (1988).

Note that the linearized network design model requires initial guess on the weighing factors, which may or may not be the optimal one. Therefore, the linearized model must be solved iteratively each time the weighing factors are revised and updated if the current solutions are different from the previous solutions. The procedure then is repeated until the solutions converge.

### V.2.3 Numerical Results and Discussion

Knowing that station 121-A had already been removed from the Snowy Range Observatory, the remaining twenty stations were then used in the model to find the optimum subset of precipitation stations to be retained. The network model was solved to select six stations (N=6) out of 20. Six stations (103-1, 106, 108-2, 119, 121, 124) which were not too close to each other were chosen as the initial solution. After numerous iterations, the best six stations did not converge; they varied from one iteration to another even though the minimum error was smaller or stayed the same. Different initial solutions for the weighing factors such as those computed by inverse distance or from previous kriging results were used and the algorithm still failed to converge.

One possible explanation for the algorithm failure was that the solutions to the optimal network design model was very sensitive to the initial stations chosen in linearization procedure. To select the best 6 out of 20 existing stations, there are  $\binom{2}{2} = 38760$  possibilities. The solution selected may be far away from the optimal one.

#### CHAPTER VI

#### SUMMARY AND RECOMMENDATION

In Chapter II, several estimation methods were used to estimate missing values and the accuracy of each method was compared with RMSE and MAE. There was no single estimation method that is uniformly superior in all circumstances. However, in the majority of the cases considered, the linear inverse distance weighing method (IDLIN) was better than all other methods through all 9 stations.

In Chapter III, detrending and deseasonalizing procedures were employed to make the time series data stationary. It was found that there was no discernable long-term trend in annual average precipitation. Both monthly sample statistics and the fitted monthly sample mean by a Fourier series were used to standardize the monthly precipitation for removing the within-year seasonal pattern in monthly precipitation time series.

In Chapter IV, the inverse distance and Gaussian smoothing weighing methods, which do not consider spatial correlation, were used to estimate the monthly average precipitation over the space. The nonstationary kriging, which considers spatial correlation, was also used to estimate the monthly precipitation. Two models were formulated by the nonstationary kriging and they were compared with the contour maps for the means and the errors. The models I and II yielded almost identical spatial patterns of monthly precipitation, but the model I had large errors in estimation. The kriging models, the inverse distance method, and Gaussian smoothing technique were also compared with the contour maps for the means and the errors. It was observed that the kriging method was superior to other two methods.

In Chapter V, two reduced precipitation networks, each retains 15 and 12 gages, are subjectively selected on the basis of spatial locations of gages, record length of gage, and aesthetic reason. The results indicated that error increases as the number of retained stations decreases. The maximum increase in error during May-September associated with the reduced networks of 15 (28% reduction in network size) and 12 (43% reduction in network size) stations, from an original 21 stations, are about 8% and 10%, respectively. However, during October-April when precipitation is most likely to be in the form of snow, the percentage error increases to a maximum of 28% for the 15-station network and 30% for the 12-station network. The area within the watershed where larger error occurs is above the Brooklyn Lake.

A model for optimal precipitation network configuration was developed. The resulting formulation was a nonlinear integer programming model to which there is no commercial software available for use. A procedure was attempted to linearize the nonlinear function and so that the well developed linear integer progamming technique can be applied. Unfortunately, the solutions from the linearization procedure were not

stable. Two possible alternative methods can be applied to solve the problem. One is to use dynamic programming approach which might be applied to find improved solutions. By dynamic programming approach, the optimization problem is divided into stages representing the number of existing stations to be retained or removed. In each stage, the state can be the list of all existing stations in the network to be retained or removed. The problem can be solved backward or forward using appropriate recursive procedure. Alternatively, recognizing the network design model has a quadratic objective function, it is possible to solve the model by embedding the quadratic programming algorithm into the branch and bound algorithm.

From the study, the following conclusions can be made:

- (1) During the months of May-September the precipitation gage number can be significantly reduced without loosing much precipitation information content in the Snowy Range watershed.
- (2) During months of snow (October-April), larger error could occur in the upper part of the watershed above Brooklyn Lake with reduced network. The accuracy on the lower portion of the watershed is not significantly affected by the reduced network.
- (3) From the above observation, it places quite a dilemma in the present operation of the precipitation network. The upper portion of the watershed area where high error occurs is generally difficult to access especially during the winter season. Currently, the gages are maintained and checked by Water Center staff on the weekly basis. To reduce labor intensiveness of maintaining the gages and to retain the precipitation accuracy one or two gages that can operate over longer period with high reliability can be installed in the remote area.

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APPENDICES

Appendix A : Contour Maps for Means and Errors



Model I, Mean

Figure 1V.4-1 Comparison of Mean Contours from Model I & II for February







Figure 1V.5-1 Contours of Difference between Model I & II for February

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Figure 1V.6-1 Comparison of Error Contours from Model I & II for February



Figure 1V.4-2 Comparison of Mean Contours from Model I & II for March







Figure IV.5-2 Contours of Difference between Model I & II for March

Model I, Error



Figure 1V.6-2 Comparison of Error Contours from Model I & II for March

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Model II, Mean



Figure IV.4-3 Comparison of Mean Contours from Model I & II for April



Error



Figure IV.5-3 Contours of Difference between Model I & II for April

Model I, Error



Figure IV.6-3 Comparison of Error Contours from Model I & II for April



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Model II, Mean



Figure IV.4-4 Comparison of Mean Contours from Model I & II for May





Error



Figure 1V.5-4 Contours of Difference between Model I & II for May





Figure IV.6-4 Comparison of Error Contours from Model I & II for May



Figure 1V.4-5 Comparison of Mean Contours from Model I & II for June





Error



Figure 1V.5-5 Contours of Difference between Model I & II for June



Figure IV.6-5 Comparison of Error Contours from Model I & II for June





Model II, Mean



Figure IV.4-6 Comparison of Mean Contours from Model I & II for July



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Error



Figure IV.5-6





Figure IV.6-6 Comparison of Error Contours from Model I & II for July



Figure IV.4-7 Comparison of Mean Contours from Model I & II for August





Figure 1V.5-7 Contours of Difference between Model I & II for August



Figure IV.6-7 Comparison of Error Contours from Model I & II for August



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Model II, Mean



Figure IV.4-8 Comparison of Mean Contours from Model I & II for September



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Figure IV.5-8 Contours of Difference between Model I & II for September

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Figure IV.6-8 Comparison of Error Contours from Model I & II for September



Figure IV.4-9 Comparison of Mean Contours from Model I & II for October





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Figure 1V.5-9 Contours of Difference between Model I & II for October



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Model II, Error



Figure IV.6-9 Comparison of Error Contours from Model I & II for October




Model II, Mean



Figure IV.4-10 Comparison of Mean Contours from Model I & II for November



Error



Figure IV.5-10 Contours of Difference between Model I & II for November

Model I, Error



Figure IV.6-10 Comparison of Error Contours from Model I & II for November





Model II, Mean



Figure 1V.4-11 Comparison of Mean Contours from Model I & II for December



Mean

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Error



Figure IV.5-11 Contours of Difference between Model I & II for December

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Model I, Error

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Figure IV.6-11 Comparison of Error Contours from Model I & II for December

Appendix B : Computer Program for Kriging

## PROGRAM KRIGING

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С	
С	THIS PROGRAM IS DOING AS (1) CALCULATES
Ċ	DISTANCES BETWEEN STATIONS WITH THE ALTITUDE.
Ċ	LATITUDE AND LONGITUDE (2)COMPUTES MEANS AND
č	VARIANCES OF INPUT VARIABLES (3) ESTIMATES THE
c	MEANS VARIANCES AND STANDARD DEVIATIONS USING
ĉ	CAUSSIAN SMOOTHING (4) STANDARDIZE THE VARIABLES
ĉ	TO BE STATIONARY (5) COMPUTES THE COVARIANCES
c	PETHEEN VARIABLES (6) AT FACH ESTIMATION (CRID)
c	DOINT ESTIMATES THE MEANS AND THE STANDARD
	DEVIATIONS USING INTERES DISTANCE DESCHTS AND
	CAUCATAN COOPULING UELCUTS FOR CONTOUR MARS (7)
C	GAUSSIAN SMOUTHING WEIGHIS FOR CONTOUR MAPS (7)
C	AFTER FINDING THE COVARIANCE FUNCTION FROM THE
С	CORRELATION MATRIX, OBTAIN THE OPTIMUM WEIGHTS
С	FROM NON-STATIONARY KRIGING (GRG2 PROGRAM IS
С	APPLIED TO THE SOLUTION PROCEDURE). THEN MONTHLY
С	PRECIPITATION IS ESTIMATED WITH OPTIMUM WEIGHTS
С	AND THE MEANS AND THE STANDARD DEVIATIONS ARE
С	OBTAINED AGAIN.
С	
С	INPUT DATA:
С	
С	X(S,T) = PRECIPITATION VARIABLE AT STATION S AT
С	MONTH T WHERE $S=1, 2,, 21$ AND $T=1, 2,, 169$
С	RAT(S) = LATITUDE OF STATION S
С	RONG(S) = LONGITUDE OF STATION S
С	ALT(S) = ALTITUDE OF STATION S
С	GRIDP(K,3) = CHOSEN K NUMBER OF ESTIMATION (GRID)
С	POINTS WITH X:Y:Z COORDINATES
С	
С	OUTPUT DATA :
C	
С	AVCONT(K) - MONTHLY AVERAGE ESTIMATED BY INVERSE
C	DISTANCE, GAUSSIAN SMOOTHING OR OPTIMUM
c	WEIGHTS AT ESTIMATION POINT k,
Ċ	k=1,2,,K
Ĉ	STDCONT(K) = STANDARD DEVIATION ESTIMATED AT
Ċ	ESTIMATION POINT k
ĉ	
ĉ	VARTARIE ·
c	
c	DTST(S S') = DISTANCE BETWEEN STATION S AND S'
ĉ	DISTN(S) = DISTANCE BETWEEN STATION S AND
0	Promition promition primition o und

С		ESTIMATION POINT AT EACH ITERATION
С	AVERMON(S,M)	- MONTHLY AVERAGE OF VARIABLE AT
С		STATION S AND A GIVEN MONTH M
С	STDERR(S,M)	= STD DEV OF " "
с	VARIANCE(S.M)	- VARIANCE OF "
c	AVHAT(S.M)	- MONTHLY AVERAGE ESTIMATED BY
c		GAUSSIAN SMOOTHING AT STATION S AND
c		A GIVEN MONTH M
c	STDHAT(S.M)	= STD DEV ESTIMATED " "
c	AC(M)	= OPTIMAL C FOR THE MEAN COMPLITED BY
c		FIBONACCI SEARCH AT A GIVEN MONTH
c c	SC(M)	= OPTIMAL C FOR THE STD DEV
c c	VC(M)	= OPTIMAL C FOR THE VARIANCE
c c		- CORRELATION BETWEEN STATION S AND S'
c c	$\operatorname{SU}(S)$	- INVERSE DISTANCE WEIGHT
	SW(S)	- CAUSSIAN SMOOTHING WEIGHT
		- TEMDODARY VARIABLE OF ESTIMATIO
	GRID(3)	POINT
	GRIDP(K, 5)	= LOCATION VARIABLE OF
	VCONTOUR(K,I)	= ESTIMATED DATA USING OPTIMUM WEIGHT
C a	RHUI(S)	= CORRELATION DEIWEEN STATION 5 AND
C		EACH ESTIMATION POINT FILLED BY THE
C		VARIOGRAM
C	RHO2(S,S')	= CORRELATION BETWEEN STATION S AND S'
С		FITTED BY THE VARIOGRAM
С	C(S,S')	= ESTIMATED LHS COVARIANCE ON KRIGING
С		SYSTEM
С	COI(S)	= " RHS " "
С	AMU(S)	= MEAN VALUE ON NONSTATIONARY KRIGING
С	AMUO	= ESTIMATED MEAN OF ESTIMATION POINT
С	TOTVAR	= ESTIMATED VARIANCE OVER THE BASIN
С	AVCONT(K)	- MONTHLY AVERAGE COMPUTED BY THE DATA
С		ESTIMATED FROM EACH WEIGHTING METHOD
C	STDCONT(K)	= STD_DEV "
С	VARCONT(K)	= VARIANCE "
С		
	implicit double	precision(a-h,o-z), integer(i-n)
	PARAMETER(itime	=169,MONTH=12,istat=21,igrid=32)
С		-
	common /coma/DI	ST(istat,istat)
	common /combl/c	(istat,istat)
	common /comb2/c	oi(istat)
	common /comb3/a	mu(istat)
	common /comb4/a	muO
	common /comc/im	on
	common /comd/di	stn(istat)
	common /comg/to	tvar
С	,	
-	DIMENSION RAT(i	stat), RONG(istat), ALT(istat)
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DIMENSION X(istat, itime), Z(istat, itime) DIMENSION AVERMON(istat, MONTH), STDERR(istat, MONTH), VARIANCE(istat, MONTH) DIMENSION avhat(istat,month), stdhat(istat,month), varhat(istat,month) & DIMENSION ac(MONTH), sc(MONTH), vc(MONTH) DIMENSION COV(istat, istat) DIMENSION sw(istat), gw(istat) DIMENSION GRID(3), GRIDP(igrid,3), VCONTOUR(igrid, itime) 1 DIMENSION RHO1(istat), rho2(istat, istat) DIMENSION avcont(igrid), stdcont(igrid), varcont(igrid) 1 DIMENSION skip(igrid) CHARACTER FILNAME\*80, NAMES(istat)\*4, NAMEM(MONTH)\*3 OPEN(UNIT=5, FILE='PREcLOC.DAT', STATUS='OLD') OPEN(uNIT=50, FILE='GRIDPNT.DAT', STATUS='OLD') OPEN(uNIT=50, FILE='WGRIDPNT.DAT', STATUS='OLD') OPEN(UNIT=51, FILE='PMO101.DAT', STATUS='OLD') OPEN(UNIT=52,FILE='PM0102.DAT',STATUS='OLD') OPEN(UNIT=53, FILE='PM01031.DAT', STATUS='OLD') OPEN(UNIT=54, FILE='PM01032.DAT', STATUS='OLD') OPEN(UNIT=55, FILE='PM0103A.DAT', STATUS='OLD') OPEN(UNIT=56, FILE='PM0106.DAT', STATUS='OLD') OPEN(UNIT=57, FILE='PM01082.DAT', STATUS='OLD') OPEN(UNIT=58, FILE='PM0108A.DAT', STATUS='OLD') OPEN(UNIT=59, FILE='PM0109.DAT', STATUS='OLD') OPEN(UNIT=60, FILE='PM01152.DAT', STATUS='OLD') OPEN(UNIT=61, FILE='PM0115A.DAT', STATUS='OLD') OPEN(UNIT=62, FILE='PM0119.DAT', STATUS='OLD') OPEN(UNIT=63,FILE='PM0120.DAT',STATUS='OLD') OPEN(UNIT=64, FILE='PM0121.DAT', STATUS='OLD') OPEN(UNIT=65, FILE='PM0121A.DAT', STATUS='OLD') OPEN(UNIT=66, FILE='PM0122.DAT', STATUS='OLD') OPEN(UNIT=67,FILE='PM0123.DAT',STATUS='OLD') OPEN(UNIT=68, FILE='PM0124.DAT', STATUS='OLD') OPEN(UNIT=69, FILE='PM0125.DAT', STATUS='OLD') OPEN(UNIT=70, FILE='PM0126.DAT', STATUS='OLD') OPEN(UNIT=71,FILE='PM0127.DAT',STATUS='OLD') OPEN(UNIT=6,FILE='krige.OUT',STATUS='UNKNOWN') READ(5,\*)((Rong(ISTA),Rat(ISTA),ALT(ISTA)), ISTA=1, istat) 1

READ(50,\*)((GRIDP(I,J),J=1,3),I=1,igrid) READ(51,\*)(X(1,K),K=1,itime) READ(52,\*)(X(2,K),K=1,itime)

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READ(53,\*)(X(3,K),K=1,itime)

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READ(54,*)(X(4,K),K=1,itime)
      READ(55,*)(X(5,K),K=1,itime)
      READ(56,*)(X(6,K),K=1,itime)
      READ(57,*)(X(7,K),K=1,itime)
      READ(58,*)(X(8,K),K=1,itime)
      READ(59,*)(X(9,K),K=1,itime)
      READ(60,*)(X(10,K),K=1,itime)
      READ(61,*)(X(11,K),K=1,itime)
      READ(62,*)(X(12,K),K=1,itime)
      READ(63,*)(X(13,K),K=1,itime)
      READ(64,*)(X(14,K),K=1,itime)
      READ(65,*)(X(15,K),K=1,itime)
      READ(66,*)(X(16,K),K=1,itime)
      READ(67,*)(X(17,K),K=1,itime)
      READ(68,*)(X(18,K),K=1,itime)
      READ(69,*)(X(19,K),K=1,itime)
      READ(70,*)(X(20,K),K=1,itime)
      READ(71,*)(X(21,K),K=1,itime)
С
      DATA NAMES/'101','102','1031','1032','103A','106'
                 '1082','108A','109','1152','115A','119',
     &
                  '120','121','121A','122','123','124',
     &
                  '125','126','127'/
     &
      DATA NAMEM /'JUN','JUL','AUG','SEP','OCT','NOV','DEC',
                   'JAN', 'FEB', 'MAR', 'APR', 'MAY'/
     æ
С
      FILNAME - 'KRIGE00'
                                   GET THE DISTANCE BETWEEN
С
                                   STATIONS
С
      CALL DISTANT (RAT, RONG, ALT)
                                   GET THE MONTHLY AVERAGE,
С
                                   THE STD DEV AND THE
С
                                   VARIANCE AT THE GIVEN
С
                                    STATION
С
      CALL AVERAGE (X, AVERMON, STDERR, VARIANCE)
                                    ESTIMATE MEAN, STD DEV AND
С
                                   VARIANCE WITH GAUSSIAN
С
                                    SMOOTHING AND OBTAIN
С
                                    THE OPTIMUM C
С
      call GAUSSSMO (avermon, ac, avhat)
      call GAUSSSMO (stderr,sc,stdhat)
      call GAUSSSMO (variance, vc, varhat)
                                    STANDARDIZE THE VARIABLES
С
      CALL STANDARD (x, avermon, stderr, Z)
                                    GET THE COVARIANCE OF
С
                                    STANDARDIZED VARIABLES
С
                                    BETWEEN TWO DIFFERENT
С
                                    STATIONS
С
      CALL CORRCOV (z,cov,const)
```

ITERATION FOR EACH MONTH DO 10 IMON = 1, MONTH IF (IMON.LE.9) THEN WRITE(FILNAME(7:7),'(I1)') IMON ELSEIF (IMON.LE.99) THEN WRITE(FILNAME(6:7), '(12)') IMON ENDIF CLOSE(UNIT=10) OPEN(10, FILE=FILNAME, STATUS='NEW') Decide the grid points DO 30 iter =1,igrid DO 31 I = 1, 3 31 grid(i) = gridp(iter,i) Get the new distances between each grid point and station S call DISTGRID (grid, rat, rong, alt) Choose the weighting method method=3 if(method.eq.1) then Get the 'IVDIS' weights CALL IDWEIGHT (sw) Estimate the means and std dev for contour line using the weights call ESTIMATE (sw,iter,avermon,stderr,avcont,stdcont) elseif(method.eq.2) then Get the weights from Gaussian Smoothing using the optimum C, then estimate the means and std dev using the weights call GAWEIGHT (ac,gw) call ESTIMATE (gw,iter,avermon,stderr,avcont,skip) call GAWEIGHT (sc,gw) call ESTIMATE (gw,iter,avermon,stderr,skip,stdcont) elseif(method.eq.3) then do 32 i=1,istat 32 amu(i) = avermon(i,imon) Get the initial weights for GRG2 CALL IDWEIGHT (const,sw)

1

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C c

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C C

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С

С

2	Get the estimated RHS and C LHS correlation matrices
	call RLHSRHO(const, rhol, rho2)
2	Get the estimated LHS and C
	RHS Covariance matrices
3	for Kriging
	CALL RLHSCOV (sw,cov,stderr,avermon,rho1,rho2)
	Get the optimum weights
U	IIOM GRG2
_	call OPIWEIRI(SW)
	PRECIPITATION using the
	ontimum weights FOR given
	month
	do 33 jimimon itime 12
	VSIM = 0
	DO 34 TI = 1. istat
	if(x(ii.ii).ne.9999.) then
	VSUM = VSUM + sw(II)*x(II,jj)
	else
	vsum=9999.
	endif
	34 continue
	33 VCONTOUR(iter,jj) = VSUM
с	Get the mean and variance C
	of estimated monthly TOTAL
с	precipitation from kriging
	call contaver
С	(vcontour,iter,avcont,stdcont,varcont)
	endif
	30 continue
С	
	do 40 $i=1, igrid$
_	40 write(10,110) gridp(1,1), gridp(1,2), $t(1) = t(1)$
С	& avcont(1),stdcont(1)
	10 continue 110 formula (20// $m$ f5 2 2 $m$ f5 2 2 $m$ f11 7 2 $m$ f11 7)
	IIU format (52(4x,15.5,5x,15.5,5x,111.7,5x,111.7))
	SIUP
~	END
с С	*****
С· С·	***************************************
c c	
c	COMPUTE THE AVERAGE AND STANDARD DEVIATION OF DATA
c.	
3	SUBROUTINE AVERAGE (X.AVERMON.STDERR.VAR)
С	
c	INPUT DATA :
č	X(S,T) - MONTHLY PRECIPITATION DATA AT STATION

С S AND AT MONTH T С С OUTPUT DATA : С AVERMON(S,M) = MONTHLY MEAN OF DATA AT STATION SС AT A GIVEN M С STDERR(S,M) = STD DEV OF DATA " С - VARIANCE OF DATA " VAR(S,M) С implicit double precision(a-h,o-z), integer(i-n) PARAMETER(itime=169, istat=21, MONTH=12) С dimension X(istat, itime), AVERMON(istat, MONTH), STDERR(istat, MONTH), VAR(istat, MONTH) & С DO 10 ISTA = 1, istat DO 20 MON -1, MONTH SSUM = 0.ACOUNT = 0. DO 40 itim = MON, itime, MONTH IF (X(ISTA, itim).NE.9999.) THEN SSUM = SSUM + X(ISTA, itim) ACOUNT = ACOUNT + 1.ELSE ENDIF 40 CONTINUE AVERMON(ISTA, MON) = SSUM / ACOUNT SUM = 0. DO 50 itim = MON, itime, MONTH IF (X(ISTA, itim).NE.9999.) THEN SUM = SUM + (X(ISTA,itim)-AVERMON(ISTA,MON))\*\*2 ENDIF CONTINUE 50 VAR(ISTA,MON)=SUM/(ACOUNT-1.) STDERR(ISTA,MON) = DSQRT(VAR(ISTA,MON)) 20 CONTINUE 10 CONTINUE RETURN END С С С COMPUTE THE CORRELATION BETWEEN TWO STATIONS С SUBROUTINE CORRCOV (SX,SCOV,const) С INPUT DATA : С - THE STANDARDIZED MONTHLY PRECIPITATION С SX(S,T)С DATA AT STATION S AND IN MONTH T

С С OUTPUT DATA : С SCOV(S,S') = CORRELATION BETWEEN STATION S AND S'С CONST - AVERAGE OF CORRELATIONS RELATED TO С STATION 108-2 С implicit double precision(a-h,o-z), integer(i-n) PARAMETER (istat=21,itime=169) dimension SX(istat, itime), SCORR(istat, istat), ۶£ SCOV(istat, istat) С DO 10 ISTA = 1, istat DO 20 JSTA = 1, istat XSUM = 0. YSUM = 0.DF = 0. DO 30 I = 1, itime IF (SX(ISTA, I).NE.9999.) THEN DF = DF + 1. XSUM = XSUM + SX(ISTA, I)YSUM = YSUM + SX(JSTA, I)ENDIF 30 CONTINUE С XMEAN = XSUM / DF YMEAN - YSUM / DF С RNUM = 0. SSX = 0.SSY = 0.DO 40 I = 1, itime IF (SX(ISTA,I).NE.9999.) THEN RNUM = RNUM + (SX(ISTA, I) - XMEAN)\*1 (SX(JSTA, I) - YMEAN) SSX = SSX + (SX(ISTA,I)-XMEAN)\*\*2 SSY = SSY + (SX(JSTA, I) - YMEAN) \*\*2ENDIF 40 CONTINUE DENUM = DSQRT(SSX\*SSY) SCORR(ISTA, JSTA) = RNUM / DENUM SCOV(ISTA,JSTA) = SCORR(ISTA,JSTA) 20 CONTINUE **10 CONTINUE** sum0=0. ccount=0. do 80 i=1,istat if(i.ne.7) then ccount=ccount+1. sum0=sum0+scov(i,7)

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endif
  80 continue
     const=sum0/ccount
     do 81 i=1,istat
      if(i.ne.7) then
       scov(i,7)=const
       scov(7,i)=const
      endif
  81 continue
     RETURN
     END
С
С
   STANDARDIZE THE DATA
С
С
     SUBROUTINE STANDARD (x, aver, stder, SZ)
С
С
     INPUT DATA :
                 - MONTHLY PRECIPITATION DATA
С
       X(S,T)
       AVER(S,M) - MONTHLY AVERAGE OF DATA AT STATION
С
С
                   S AND IN A GIVEN MONTH M
       STDER(S,M) = STANDARD DEVIATION OF DATA AT STATION S
С
                   AND IN A GIVEN MONTH M
С
С
С
     OUTPUT DATA :
                 - STANDARDIZED DATA AT STATION S AND IN
С
       SZ(S,T)
С
                   MONTH T
С
     implicit double precision(a-h,o-z),integer(i-n)
     PARAMETER(istat=21,MONTH=12,itime=169)
     dimension x(istat,itime),SZ(istat,itime),
          aver(istat,MONTH),stder(istat,MONTH)
    δ.
С
     DO 10 ISTA = 1, istat
      DO 20 MON = 1, MONTH
       DO 30 I - MON, itime, 12
        IF (x(ISTA,I).NE.9999.) THEN
        sz(ista,i)=(x(ista,i)-aver(ista,mon))/
                   stder(ista,mon)
    7
        else
         sz(ista,i)=9999.
        ENDIF
       CONTINUE
   30
  20 CONTINUE
   10 CONTINUE
     RETURN
     END
```

```
С
C ******
С
   GET THE DISTANCE BETWEEN TWO STATIONS USING
С
   THE LATITUDE(MILE), LONGITUDE(MILE) AND ALTITUDE(MILE)
С
С
   OF EACH STATION.
С
     SUBROUTINE DISTANT(SLAT, SLONG, SALT)
С
С
     INPUT DATA :
                - LATITUDE OF STATION S
С
       SLAT(S)
                - LOGITUDE OF STATION S
       SLONG(S)
С
       SALT(S)
                - ALTITUDE OF STATION S
С
С
     OUTPUT DATA :
С
       DIST(S,S') = DISTANCE BETWEEN STATION S AND S'
С
С
      implicit double precision(a-h,o-z),integer(i-n)
      PARAMETER (istat=21)
С
      common /coma/disT(istat,istat)
      dimension SLAT(istat), SLONG(istat), SALT(istat)
С
      DO 44 I = 1, istat
       DO 33 J = 1, istat
       RATIT = ABS(SLAT(I)-SLAT(J))
        RONGIT = ABS(SLONG(I) - SLONG(J))
        ALTIT = ABS(SALT(I)-SALT(J))/5280.
        DIST(I,J) = DSQRT(RATIT**2+RONGIT**2+ALTIT**2)
   33
       CONTINUE
   44 CONTINUE
С
     RETURN
     END
С
С
   GET THE DISNCE BETWEEN EXISTING STATION AND THE
С
   ESTIMATION POINT
С
С
     subroutine distgrid (grid, rat, rong, alt)
с
     INPUT DATA :
С
      GRID(X, Y, Z) = THE COORDINATES OF EACH ESTIMATION
С
                   POINT
С
                 = LATITUDE OF STATION S
С
      RAT(S)
                 - LOGITUDE OF STATION S
С
      RONG(S)
```

```
- ALTITUDE OF STATION S
С
     ALT(S)
С
С
    OUTPUT DATA :
                 - DISTANCE BETWEEN A GIVEN ESTIMATION
С
      DISTN(S)
                  POINT AND STATION S
С
С
     implicit double precision(a-h,o-z),integer(i-n)
    parameter(istat=21)
     common /comd/distn(istat)
     dimension grid(3), rat(istat), rong(istat), alt(istat)
С
     do 10 ista=1,istat
     a=grid(1)-rong(ista)
     b=grid(2)-rat(ista)
     c=(grid(3)-alt(ista))/5280.
  10 distn(ista)=dsqrt(a**2+b**2+c**2)
     return
     end
С
С
   CALCULATE THE INVERSE DISTANCE WEIGHT
С
С
     subroutine idweight (w)
С
     INPUT DATA :
С
               = DISTANCE BETWEEN STATION S AND THE
С
      DISTN(S)
                 ESTIMATION POINT
С
С
     OUTPUT DATA :
С
               - INVERSE DISTANCE WEIGHT OF STATION S
С
      W(S)
С
     implicit double precision(a-h,o-z),integer(i-n)
     parameter(istat=21)
     common /comd/distn(istat)
     dimension w(istat)
С
     dsum=0.
     do 11 ista=1,istat
  11 dsum=dsum+l./distn(ista)
     do 12 ista=1,istat
      w(ista)=(1./distn(ista))/dsum
  12 continue
     return
     end
С
```

```
С
    COMPUTE THE CORRELATION USING THE CORRELATION FUNCTION
С
С
      subroutine RLHSRHO(const,rho1,rho2)
С
          -----
      INPUT DATA :
С
                    - DISTANCE BETWEEN STATION S AND S'
        DIST(S,S')
С
                    - DISTANCE BETWEEN STATION S AND
С
        DISTN(S)
                       ESTIMATION POINT
С
С
С
      OUTPUT DATA :
                     - ESTIMATED CORRELATION BETWEEN STATION
        RHO1(S)
С
                       S AND ESTIMATION POINT
С
                    = ESTIMATED CORRELATION BETWEEN STATION
С
        RHO2(S,S')
                       S AND S'
С
С
      implicit double precision(a-h,o-z),integer(i-n)
      parameter(istat=21)
      common /coma/dist(istat,istat)
      common /comd/distn(istat)
      dimension rhol(istat), rho2(istat, istat)
С
      do 10 ista=1,istat
       if(distn(ista).eq.0.) then
        rhol(ista)=1.
       else
        if(ista.ne.7) then
с
        rhol(ista)=.927988-.026262*distn(ista)
        else
                                   FOR STATION 108-2
С
         rhol(ista)=const
        endif
       endif
С
       do ll jsta=l,istat
        if(dist(ista,jsta).eq.0.) then
         rho2(ista,jsta)=1.
        else
         if(ista.ne.7.or.jsta.ne.7) then
с
          rho2(ista,jsta)=.927988-.026262*dist(ista,jsta)
         else
                                   FOR STAITON 108-2
С
          rho2(ista,jsta)=const
         endif
        endif
   11 continue
   10 continue
```

return end

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Cxx	× × × × × × × × × × × × × × × × × × ×	*****			
6××	****	~~~~~			
с С С	COMPUTE THE EL FUNCTION OF KR	EMENTS IN RIGHT HAND SIDE OF THE OBJECT IGING			
С					
	subroutine r	lhscov(w,stder,aver,rhol,rho2)			
С					
С	INPUT DATA :				
С	W(S)	= INVERSE DISTANCE WEIGHT			
C	STDER(S,M)	= STANDARD ERROR OF DATA AT STATION S			
c		AND IN A GIVEN MONTH M			
C	AVER(S,M)	A CIVEN MONTH M			
C	DUO1(C)	A GIVEN MONIA M - CODDELATION BETHERN STATION S AND			
C C	KHOI(5)	FSTIMATION POINT			
c	RH02(S S')	= CORRELATION BETWEEN STATION S AND S'			
c	TMON	= A GIVEN MONTH			
č					
C	OUTPUT DATA	:			
С	C(S,S')	= COVARIANCE BETWEEN STATION S AND S'			
С	COI(s)	- COVARIANCE BETWEEN STATION S AND			
С		ESTIMATION POINT			
С	AMUO	- ESTIMATED MEAN OF ESTIMATION POINT			
С	TOTVAR	= ESTIMATED VARIANCE ESTIMATION POINT			
С					
	implicit dou	ible precision(a-n,o-z), integer(1-n)			
	parameter(istat=21)				
	common /combl/c(lstat,lstat)				
	common /comm	$\frac{1}{2}$			
		/imon			
		/totvar			
	dimension W	(istat).stder(istat,12).aver(istat,12).			
	1 rł	nol(istat), rho2(istat, istat)			
с	-				
-	sum1=0.				
	sum2=0.				
	sum3=0.				
	do 10 ista <del>-</del> 1	l,istat			
	<pre>suml=suml+w(ista)*aver(ista,imon)</pre>				
	sum3=sum3+v	v(ista)*stder(ista,imon)**2			
	10 sum2=sum2+v	v(ista)*stder(ista,imon)			
	do ll ista=1	L,istat			
	do 12 jsta-	=1,1stat			
	12 c(ista,js	ca)=stder(1sta,1mon)*stder(]sta,1mon)*			

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rho2(ista,jsta)
    1
  11 coi(ista)=stder(ista,imon)*sum2*rhol(ista)
      amu0-sum1
      totvar=sum3
     return
     end
с
С
   CALCULATE THE OPTIMUM WEIGHT BY KRIGING
С
С
     subroutine optweiht(xx)
С
      INPUT DATA :
С
                  INVERSE DISTANCE WEIGHT OF STATION S
       XX(S)
С
С
      OUPUT DATA :
С
                  = OPTIMUM WEIGHT OF STATION S
С
       XX(S)
С
      SUBROUTINE 'GRGSUB' IS DEVELOPED BY LASDEN (1979)
С
      ALL VARIABLES USED FOR 'GRGSUB' CAN BE REFERRED
С
      FROM GRG2 USER'S GUIDE
С
С
      implicit double precision(a-h,o-z), integer(I-n)
      parameter(istat=21)
      integer*4 ncore, nnvars, nfun, maxbas, maxhes
     ~common /combl/c(istat,istat)
      common /comb2/coi(istat)
      common /comb3/amu(istat)
      common /comb4/amu0
      common /comc/imon
      common /comg/totvar
      logical maxim, inprnt, otprnt
      dimension defaul(19),z(15000),ramcon(3),ramvar(istat),
               nonbas(istat),redgr(istat),fcns(3),
     1
                inbind(3),blvar(istat),buvar(istat),
     1
               blcon(3), bucon(3), rmults(3),
     1
                ttitle(19),xx(istat)
     1
      data ncore /15000/
      data nnvars, nfun, maxbas, maxhes /21,3,3,21/
      data ramcon /8hOBJFUNCN,8hUNBIASED,8hSUMOFWGT/
      data ramvar /8hSTAT0101,8hSTAT0102,8hSTAT1031,
                  8hSTAT1032,8hSTAT103A,8hSTAT0106,
     1
                  8hSTAT1082,8hSTAT108A,8hSTAT0109,
     1
                   8hSTAT1152,8hSTAT115A,8hSTAT0119,
     1
                   8hSTAT0120,8hSTAT0121,8hSTAT121A,
     1
                   8hSTAT0122,8hSTAT0123,8hSTAT0124,
     1
                   8hSTAT0125,8hSTAT0126,8hSTAT0127/
     1
```

```
data blvar/istat*0.0/,buvar/istat*1.0/
      data blcon /0.0,0.0,0.0/, bucon /1.0E3,0.0,0.0/
      data ttitle /19*4h
c
      inprnt=.true.
      otprnt=.true.
С
      do 10 i=1,19
       defaul(i)=1.0
   10 continue
C
      nnobj=1
      call GRGSUB(inprnt, otprnt, ncore, nnvars, nfun, maxbas,
     1
                 maxhes, nnobj, ttitle, blvar, buvar, blcon,
     1
                 bucon, defaul, fpnewt, fpinit, fpstop, fpspiv,
     1
                 pphlep,nnstop,iitllm,llmser,iipr,iipn4,
     1
                 iipn5,iipn6,iiper,iidump,iiquad,lderiv,
     1
                 mmodcg, ramcon, ramvar, xx, fcns, inbind,
     1
                 rmults,nonbas,redgr,nbind,nnonb,inform,z)
С
      write(10,200) (xx(j),j=1,nnvars)
С
      write(10,210) (fcns(i), i=1, nfun)
С
      write(10,220) (nonbas(i),redgr(i),i=1,nnonb)
С
      write(10,230) (inbind(i),rmults(i),i=1,nbind)
С
      write(10,240) nnonb, nbind
с
      write(10,250) inform
С
  200 format(///lhl,lx,'FINAL VALUES OF DECISION
            VARIABLES: '/5(5(5X,E12.5)/))
     1
  210 format(/lx,'FINAL VALUES OF CONSTRAINTS AND
     1
            OBJECTIVE: '/5(5X, E12.5))
  220 format(/1x,'REDUCED GRADIENT:'/5(5(5X,I3,2X,E12.5)/))
  230 format(/1x, 'VALUES OF LAGRANGE MULTIPLIERS:'/
            5(5X,I3,2X,E12.5))
     1
  240 format(/lx,'NUMBER OF STRUCTURAL NONBASIC VARIABLES:',
            5X, I5/1X, 'NUMBER OF BINDING CONSTRAINTS: '5X, I5)
     1
  250 format(/1x,'REASON FOR TERMINATION:',5X,I5)
      return
      end
С
с
С
    COMPUTE THE OBJECT AND CONSTRAINTS FUNCTION
С
     subroutine GCOMP(g,x)
с
С
     INPUT DATA :
С
       G(3)
                  - TOTAL FUNCTIONS USED IN THE PROBLEM
С
       X(S)
                  - UPDATED INPUT SOLUTION
```

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```

```
С
     OUTPUT DATA :
С
                 - UPDATED OUTPUT SOLUTION
С
       X(S)
С
     SUBROUTINE GCOMP IS DEVELOPED BY LASDEN (1979)
С
С
     implicit double precision(a-h,o-z), integer(i-n)
     parameter(istat=21)
     common /combl/c(istat,istat)
     common /comb2/coi(istat)
     common /comb3/amu(istat)
     common /comb4/amu0
     common /comg/totvar
     dimension g(3), x(istat)
С
     f1=0.
     f2=0.
     f3=0.
      f4=0.
      do 10 i=1,istat
      do 11 j=1,istat
      fl=fl+x(i)*x(j)*c(i,j)
   11
      f3=f3+x(i)*amu(i)
       f_{4=f_{4+x(i)}}
   10 f_{2=f_{2+x(i)*coi(i)}}
      g(1)=f1-2.*f2+totvar
      g(2)=f3-amu0
      g(3)=f4-1.
      return
      end
С
С
    ESTIMATE THE MEANS AND THE VARIANCE USING GAUSSIAN
С
    SMOOTHING
С
С
      SUBROUTINE GAUSSSMO (X,GC,GX)
С
      INPUT DATA :
С
                  - THE MEAN OR THE VARIANCE OF DATA AT
С
        X(S,M)
                    STATION S AND IN MONTH M
С
 С
      OUTPUT DATA :
 С
                  - OPTIMAL SMOOTHING FACTOR IN MONTH M
 С
        GC(M)
                  - ESTIMATED MEANS OR VARIANCE BY
 С
        GX(S,M)
                    GAUSSIAN SMOOTHING
 С
 С
      implicit double precision(a-h,o-z),integer(i-n)
```

```
PARAMETER (istat=21,MONTH=12)
    common /coma/DIST(iSTAT,iSTAT)
    common /comf/PX(iSTAT)
    dimension X(iSTAT, MONTH), GX(iSTAT, MONTH),
        XHAT(iSTAT),GC(MONTH)
    &
    DO 10 MON = 1, MONTH
      DO 20 ISTA = 1, iSTAT
       PX(ISTA) = X(ISTA, MON)
      CALL FIBON (PC)
      CALL GAUSSHAT (PC, XHAT)
      GC(MON) = PC
       print*,pc
     DO 30 ISTA = 1, iSTAT
     GX(ISTA,MON) = XHAT(ISTA)
  30
  10 CONTINUE
     RETURN
     END
C *****
CALCULATE THE GAUSSIAN SMOOTHING WEIGHT
     SUBROUTINE GAUSSHAT (CC, RESULT)
     INPUT DATA :
                - UPDATED OPTIMAL SMOOTHING FACTOR
       CC
       DIST(S,S') = DISTANCE BETWEEN STATION S AND S'
```

С

С

С

С С

С

С

С С

С С

С

С С

С

С С

С

```
OUTPUT DATA :
 RESULT(S) - UPDATED ESTIMATED DATA OF STATION S
implicit double precision(a-h,o-z),integer(i-n)
PARAMETER (istat=21)
common /coma/DIST(iSTAT,iSTAT)
common /comf/PX(iSTAT)
dimension RESULT(iSTAT), F(iSTAT), W(iSTAT)
```

```
DO 10 ISTA = 1, iSTAT
   DO 11 J = 1, iSTAT
    F(J) = DEXP((-DIST(ISTA,J)**2)/(2.*CC))
11 CONTINUE
   FSUM = 0.
   DO 20 I = 1, iSTAT
20 FSUM = FSUM + F(I)
```

```
DO 30 I = 1, iSTAT
  30 W(I) = F(I) / FSUM
     WSUM = 0.
     DO 40 I = 1, iSTAT
  40 WSUM = WSUM + W(I)*PX(I)
     RESULT(ISTA) = WSUM
  10 CONTINUE
     RETURN
     END
С
    SEARCH THE OPTIMAL SMOOTHING FACTOR BY FIBONACCI
С
С
    SEARCH METHOD
С
     SUBROUTINE FIBON (GC)
С
С
     INPUT DATA :
С
       PX(S)
                 = UPDATED ESTIMATED DATA
С
С
     OUTPUT DATA :
                - UPDATED SMOOTHING FACTOR
С
       GC
С
     implicit double precision(a-h,o-z),integer(i-n)
     common /comf/px(21)
     DIMENSION F(500)
     DATA A, B, ALPHA/0., 10., 0.01/
С
     BB-B
     AA=A
     UNCIV-B-A
     FNPR=UNCIV/ALPHA
     F(1) = 1.0
     F(2) = 1.0
     DO 5 I=3,500
     F(I)=F(I-1)+F(I-2)
     IF(FNPR-F(I))6,6,5
   5 CONTINUE
   6 IFN=I
     IFPRT=I
   15 P1=AA+(F(IFN-2)/F(IFN))*UNCIV
     P2=AA+(F(IFN-1)/F(IFN))*UNCIV
     CALL FUNC(P1, ERR1)
     CALL FUNC(P2, ERR2)
     IF(ERR1-ERR2)10,11,12
   12 AA=P1
     GOTO 13
   10 BB=P2
     GOTO 13
```

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11 BB=P2
    AA=P1
  13 UNCIV-BB-AA
    IFN=IFN-1
    IF(IFN-3)20,15,15
  20 GC=BB
     RETURN
     END
С
С
    CALCULATE THE ERROR CONCERNING TO FIND THE
С
    OPTIMAL SMOOTHING FACTOR
С
С
     SUBROUTINE FUNC (CC, ERR)
С
С
     INPUT DATA :
      CC
               - UPDATED SMOOTHING FACTOR
С
               - OBSERVED DATA AT STATION S
С
      PX(S)
               - ESTIMATED DATA AT STATION S
С
      PXH(S)
С
     OUTPUT DATA :
С
               - THE ERROR CONCERNING THE ESTIMATES
С
      ERR
С
     implicit double precision(a-h,o-z),integer(i-n)
     PARAMETER (istat=21)
     common /comf/PX(iSTAT)
     dimension PXH(iSTAT)
С
     CALL GAUSSHAT (CC, PXH)
     ESUM = 0.
     DO 20 I = 1, iSTAT
      ESUM = ESUM + (PX(I) - PXH(I)) **2
  20 CONTINUE
     ERR = ESUM
     RETURN
     END
С
C *****
с
   CALCULATE THE MEAN AND THE ERROR OF THE ESTIMATED
С
   DATA WITH OPTIMUM WEIGHT
С
С
     SUBROUTINE CONTAVER (X, ITER, AVERMON, STDERR, VAR)
С
     INPUT DATA :
С
               - THE ESTIMATED DATA AT ESTIMATION
      X(K,T)
С
```

POINT K AND IN MONTH T С - REPRESENTS ITER-TH ESTIMATION POINT ITER С С OUTPUT DATA : С AVERMON(K) - MEAN OF ESTIMATED DATA AT ESTIMATION С POINT K С - STANDARD ERROR OF DATA AT ESTIMATION С STDERR(K) POINT K С - VARIANCE OF DATA AT ESTIMATION POINT K С VAR(K) С implicit double precision(a-h,o-z), integer(i-n) common /comc/imon PARAMETER(itime=169,igrid=32) С dimension X(igrid, itime), AVERMON(igrid), & STDERR(igrid), VAR(igrid) С SSUM = 0.ACOUNT = 0. DO 40 ITIM = IMON, ITIME, 12IF (X(ITER, ITIM).NE.9999.) THEN SSUM = SSUM + X(ITER, ITIM)ACOUNT = ACOUNT + 1.ELSE ENDIF CONTINUE 40 AVERMON(ITER) = SSUM / ACOUNT SUM = 0. DO 50 ITIM - IMON, ITIME, 12 IF (X(ITER, ITIM).NE.9999.) THEN SUM = SUM + (X(ITER, ITIM) - AVERMON(ITER))\*\*2 ENDIF 50 CONTINUE VAR(ITER)=SUM/(ACOUNT-1.) STDERR(ITER) = DSQRT(VAR(ITER)) RETURN END С C \*\*\*\*\*\* С CALCULATE THE GAUSSIAN SMOOTHING WEIGHT BY THE OPTIMAL С С SMOOTHING FACTOR С subroutine GAWEIGHT (gc,gw) С INPUT DATA : С - OPTIMAL SMOOTHING FACTOR IN MONTH M С GC(M) С

```
С
     OUTPUT DATA :
                  - GAUSSIAN SMOOTHING WEIGHT AT STATION S
С
       GW(S)
С
     implicit double precision(a-h,o-z),integer(i-n)
     parameter(istat=21)
     common /comc/imon
     common /comd/distn(istat)
     dimension gw(istat), f(istat), gc(12)
С
     cc=gc(imon)
     do 10 ista=1,istat
      f(ista)=dexp((-distn(ista)**2)/(2.*cc))
   10 print*, f(ista), distn(ista), cc
      fsum=0.
      do 20 ista=1,istat
   20 fsum=fsum+f(ista)
      do 30 ista=1,istat
   30 gw(ista)=f(ista)/fsum
      return
      end
С
с
      ESTIMATE THE MEANS AND STANDARD ERROR USING THE GIVEN
С
С
      WEIGHT
С
      subroutine ESTIMATE
                (w,iter,avermon,stderr,avhat,stdhat)
С
      INPUT DATA :
С
                    - THE WEIGHT AT STATION S
С
        W(S)
        AVERMON(S,M) = THE OBSERVED MEAN AT STATION S AND
С
                      IN MONTH M
С
                    THE OBSERVED STANDARD ERROR
        STDERR(s,M)
С
С
С
      OUTPUT DATA :
                    - ESTIMATED MEAN AT ESTIMATION POINT K
С
        AVHAT(K)
                    - ESTIMATED STANDARD ERROR AT
        STDHAT(K)
С
                      ESTIMATION POINT K
С
С
      implicit double precision(a-h,o-z),integer(i-n)
      parameter(istat=21,igrid=32,month=12)
      common /comc/imon
      dimension w(istat), avermon(istat, month),
                stderr(istat,month),
     1
                avhat(igrid), stdhat(igrid)
     1
 С
```

avsum=0.

```
stdsum=0.
do 10 ista=1,21
  avsum=avsum+w(ista)*avermon(ista,imon)
10 stdsum=stdsum+w(ista)*stderr(ista,imon)
  avhat(iter)=avsum
```

```
avnat(iter)=avsum
stdhat(iter)=stdsum
return
end
```

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